

Detailed Member Calculations

Units: N&mm

Regulation: ASCE 41-17

Calculation No. 1

- wall W1, Floor 1
- Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)
- Analysis: Uniform +X
- Check: Shear capacity VR_d
- Edge: Start
- Local Axis: (2)



- Start Of Calculation of Shear Capacity for element: wall W1 of floor 1
- At local axis: 2
- Integration Section: (a)
- Section Type: rcrws

Constant Properties

- Knowledge Factor, $\gamma = 0.85$
- Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.
- Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17
- Consequently:
- New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$
- New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
- Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$
 Total Height, $H_{tot} = 3000.00$
 Edges Width, $W_{edg} = 250.00$
 Edges Height, $H_{edg} = 600.00$
 Web Width, $W_{web} = 250.00$
 Cover Thickness, $c = 25.00$
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou,min} = l_b/l_d \geq 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $e_{fu} = 0.01$
 Number of directions, $N_{oDir} = 1$
 Fiber orientations, $b_i: 0.00^\circ$
 Number of layers, $N_L = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-
 Bending Moment, $M_a = 4.1575044E-011$
 Shear Force, $V_a = 8.1671580E-014$
 EDGE -B-
 Bending Moment, $M_b = 1.9900347E-010$
 Shear Force, $V_b = -8.1671580E-014$
 BOTH EDGES
 Axial Force, $F = -31849.263$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $A_{sl,t} = 0.00$
 -Compression: $A_{sl,c} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $A_{sl,ten} = 2368.761$
 -Compression: $A_{sl,com} = 2368.761$
 -Middle: $A_{sl,mid} = 1608.495$
 Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.20$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 766882.52$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d = 766882.52$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 348603.471$
 $M_u/V_u - l_w/2 = 384.0515 > 0$
 = 1 (normal-weight concrete)
 $f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $M_u = 4.1575044E-011$
 $V_u = 8.1671580E-014$
 $N_u = 31849.263$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs1 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs2 = 157079.633 is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs2 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs3 = 0.00 is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

Vs3 has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$$f = 0.95, \text{ for fully-wrapped sections}$$

$$w_f/s_f = 1 \text{ (FRP strips adjacent to one another).}$$

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(a, \dots)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

$$\text{orientation 1: } a_1 = b_1 + 90^\circ = 90.00$$

$$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|), \text{ with:}$$

$$\text{total thickness per orientation, } t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$$

$$d_{fv} = d \text{ (figure 11.2, ACI 440)} = 208.00$$

$$f_{fe}((11-5), \text{ACI 440}) = 259.312$$

$$E_f = 64828.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.01$$

$$\text{From (11-11), ACI 440: } V_s + V_f \leq 1.9929E+006$$

$$b_w = 3000.00$$

End Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 2

wall W1, Floor 1

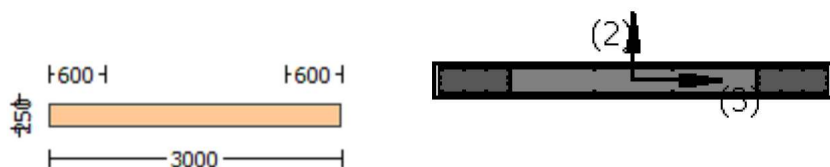
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ_u)

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcwrs

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

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Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

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Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou}, \min \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $NoDir = 1$

Fiber orientations, $bi: 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = -3.7865323E-029$

EDGE -B-

Shear Force, $V_b = 3.7865323E-029$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2865.133$

-Compression: $As_{c,com} = 2865.133$

-Middle: $As_{mid} = 0.00$

(According to 10.7.2.3 As_{mid} is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 1.77991$

Member Controlled by Shear ($V_e/V_r > 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$ with

$M_{pr1} = \text{Max}(Mu_{1+}, Mu_{1-}) = 7.6379E+009$

$Mu_{1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+}, Mu_{2-}) = 7.6379E+009$

$Mu_{2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 1.1557999E-005$

$M_u = 6.9435E+009$

with full section properties:

$b = 250.00$

$d = 2957.00$

$d' = 43.00$

$v = 0.00112784$

$N = 27514.027$

$f_c = 33.00$

ϕ_c (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.0035$

ϕ_{we} ((5.4c), TBDY) = $a s_e * \phi_{sh,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$

where $\phi_f = a f^* p f^* f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\phi_{fx} = 0.00$

$a f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $p f = 2 t f / b w = 0.008128$

$b w = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$\phi_{fy} = 0.00$

$a f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $p f = 2 t f / b w = 0.00067733$

$b w = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase_1 \cdot A_{col1} + ase_2 \cdot A_{col2} + ase_3 \cdot A_{web}) / A_{sec} = 0.00$
 $ase_1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase_2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase_3 = 0$ (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$f_{ywe} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 c = confinement factor = 1.00246

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lo,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/lb, min = 1.00$
 $su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/ld = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl, ten / (b * d) * (fs1 / fc) = 0.0815606$
 $2 = Asl, com / (b * d) * (fs2 / fc) = 0.0815606$
 $v = Asl, mid / (b * d) * (fsv / fc) = 0.00$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b * d) * (fs1 / fc) = 0.10841651$
 $2 = Asl, com / (b * d) * (fs2 / fc) = 0.10841651$
 $v = Asl, mid / (b * d) * (fsv / fc) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

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$v < vs, y2$ - LHS eq.(4.5) is satisfied

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$su (4.9) = 0.06369787$

$Mu = MRc (4.14) = 6.9435E+009$

$u = su (4.1) = 1.1557999E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Mu1-

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.1645191E-005$$

$$M_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\phi_{co} (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{co}) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.0035$$

$$\phi_{we} ((5.4c), TBDY) = a_{se} * \phi_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = a_f * \phi_{pf} * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$a_{se} ((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$a_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{psh,min} = \text{Min}(\phi_{psh,x}, \phi_{psh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{psh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{psh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$h_2 = 600.00$
 $As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,x} (web) = (As_3 * h_3 / s_3) / Ac = 0.00188496$
 $h_3 = 1800.00$
 $As_3 = Astir_3 * ns_3 = 0.00$
 No stirups, $ns_3 = 2.00$

$psh,y = ps_1,y + ps_2,y + ps_3,y = 0.0010472$
 $ps_{1,y} (column\ 1) = (As_1 * h_1 / s_1) / Ac = 0.0005236$
 $h_1 = 250.00$
 $As_1 = Astir_1 * ns_1 = 157.0796$
 No stirups, $ns_1 = 2.00$
 $ps_{2,y} (column\ 2) = (As_2 * h_2 / s_2) / Ac = 0.0005236$
 $h_2 = 250.00$
 $As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,y} (web) = (As_3 * h_3 / s_3) / Ac = 0.00$
 $h_3 = 250.00$
 $As_3 = Astir_3 * ns_3 = 157.0796$
 No stirups, $ns_3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fy_{we} = 694.45$
 $f_{ce} = 33.00$

From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 833.34$
 $fy_1 = 694.45$
 $su_1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su_1 = 0.4 * esu_{1,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = fs = 694.45$

with $Es_1 = Es = 200000.00$

$y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 833.34$
 $fy_2 = 694.45$
 $su_2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su_2 = 0.4 * esu_{2,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,

For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2 / 1.2$, from table 5.1, TBDY.

y_2, sh_2, ft_2, fy_2 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = fs = 694.45$

with $Es_2 = Es = 200000.00$

$y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v , sh_v, ft_v, fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.0815606$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.0815606$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.01752837$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.10841651$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.10841651$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.02330004$

Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.07070831$
 $Mu = MRc (4.14) = 7.6379E+009$
 $u = su (4.1) = 1.1645191E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 1.1557999E-005$
 $Mu = 6.9435E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $we ((5.4c), TBDY) = ase * sh_{min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.00$
 where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$

From EC8 A4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
effective stress from (A.35), $ff,e = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff,e = 958.8339$

$R = 40.00$
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/A_{sec} = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$ase3 = 0$ (grid does not provide confinement)
 $psh,min = Min(psh,x, psh,y) = 0.0010472$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3*ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $f_{ywe} = 694.45$
 $f_{ce} = 33.00$

From (5A.5), TBDY, TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, \min = lb/ld = 1.00$
 $su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, \min = lb/lb, \min = 1.00$
 $su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, \min = lb/ld = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl, \text{ten} / (b * d) * (fs1 / fc) = 0.0815606$
 $2 = Asl, \text{com} / (b * d) * (fs2 / fc) = 0.0815606$
 $v = Asl, \text{mid} / (b * d) * (fsv / fc) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, \text{ten} / (b * d) * (fs1 / fc) = 0.10841651$
 $2 = Asl, \text{com} / (b * d) * (fs2 / fc) = 0.10841651$
 $v = Asl, \text{mid} / (b * d) * (fsv / fc) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.06369787$$

$$M_u = M_{Rc}(4.14) = 6.9435E+009$$

$$u = s_u(4.1) = 1.1557999E-005$$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of M_{u2} -

Calculation of ultimate curvature κ_u according to 4.1, Biskinis/Fardis 2013:

$$\kappa_u = 1.1645191E-005$$

$$M_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\alpha_{co}(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \kappa_u: \kappa_u^* = \text{shear_factor} * \text{Max}(\kappa_u, \kappa_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \kappa_u = 0.0035$$

$$\omega_{se}((5.4c), TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = \alpha_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$f_y = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,y (web) = (As3*h3/s_3)/Ac = 0.00
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirrups, ns3 = 0.00

Asec = 750000.00

s_1 = 100.00

s_2 = 100.00

s_3 = 200.00

fywe = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463

c = confinement factor = 1.00246

y1 = 0.0025

sh1 = 0.008

ft1 = 833.34

fy1 = 694.45

su1 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = fs = 694.45

with Es1 = Es = 200000.00

y2 = 0.0025

sh2 = 0.008

ft2 = 833.34

fy2 = 694.45

su2 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $es_{2_nominal} = 0.08$,
 For calculation of $es_{2_nominal}$ and y_2 , sh_2, ft_2, fy_2 , it is considered
 characteristic value $fs_{y2} = fs_2/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 694.45$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/d = 1.00$
 $suv = 0.4 \cdot es_{v_nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $es_{v_nominal} = 0.08$,
 considering characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY
 For calculation of $es_{v_nominal}$ and y_v , sh_v, ft_v, fy_v , it is considered
 characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_v = fs = 694.45$
 with $Es_v = Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs_1 / fc) = 0.0815606$
 $2 = Asl, com / (b \cdot d) \cdot (fs_2 / fc) = 0.0815606$
 $v = Asl, mid / (b \cdot d) \cdot (fs_v / fc) = 0.01752837$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs_1 / fc) = 0.10841651$
 $2 = Asl, com / (b \cdot d) \cdot (fs_2 / fc) = 0.10841651$
 $v = Asl, mid / (b \cdot d) \cdot (fs_v / fc) = 0.02330004$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.07070831$
 $Mu = MRc (4.14) = 7.6379E+009$
 $u = su (4.1) = 1.1645191E-005$

 Calculation of ratio lb/d

Adequate Lap Length: $lb/d \geq 1$

 Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot fc' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$
 $Mu / Vu - lw/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $fc' = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$

lw = 3000.00

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

d = 1440.00

$A_v = 157079.633$

s = 200.00

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

f = 0.95, for fully-wrapped sections

wf/sf = 1 (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / N_{\text{Dir}} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

bw = 250.00

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

h = 250.00

d = 2400.00

lw = 3000.00

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

d = 480.00

$A_v = 157079.633$

$s = 100.00$
 $f_y = 555.56$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 628323.557$ is calculated for web, with:
 $d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 555.56$
 V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_f ((11-3)-(11.4), \text{ACI 440}) = 1.5581\text{E}+006$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 $\ln(11.3) \sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = N_L * t / \text{NoDir} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00
 $f_{fe} ((11-5), \text{ACI 440}) = 259.312$
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 2.2897\text{E}+006$
 $b_w = 250.00$

 End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At local axis: 3

 Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At Shear local axis: 2
 (Bending local axis: 3)
 Section Type: rcrcws

Constant Properties

 Knowledge Factor, $\phi = 0.85$
 Mean strength values are used for both shear and moment calculations.
 Consequently:
 New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$
 New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 #####
 Note: Especially for the calculation of moment strengths,
 the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
 New material: Steel Strength, $f_s = 1.25 * f_{sm} = 625.00$
 #####
 Total Height, $H_{tot} = 3000.00$
 Edges Width, $W_{edg} = 250.00$
 Edges Height, $H_{edg} = 600.00$
 Web Width, $W_{web} = 250.00$
 Cover Thickness, $c = 25.00$
 Mean Confinement Factor overall section = 1.00246
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou, \min} \geq 1$)
 FRP Wrapping Data
 Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $NoDir = 1$

Fiber orientations, $b_i = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 1.1832914E-030$

EDGE -B-

Shear Force, $V_b = -1.1832914E-030$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2368.761$

-Compression: $As_{l,com} = 2368.761$

-Middle: $As_{l,mid} = 0.00$

(According to 10.7.2.3 $As_{l,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 0.21417884$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$

with

$M_{pr1} = \text{Max}(\mu_{u1+}, \mu_{u1-}) = 3.9637E+008$

$\mu_{u1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$\mu_{u1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(\mu_{u2+}, \mu_{u2-}) = 3.9637E+008$

$\mu_{u2+} = 3.0844E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$\mu_{u2-} = 3.9637E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 0.00018625$

$\mu_u = 3.0844E+008$

with full section properties:

$b = 3000.00$

$d = 208.00$

$d' = 42.00$

$v = 0.00176372$

$N = 27514.027$

$f_c = 25.00$

ϕ_o (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_o) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.0035$

we ((5.4c), TBDY) = $ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$
where $f = af * pf * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
effective stress from (A.35), $ff_e = 757.2164$

$f_y = 0.00$
 $af = 0.00$
 $b = 3000.00$
 $h = 250.00$
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff_e = 954.4864$

$R = 40.00$
Effective FRP thickness, $tf = NL * t * \text{Cos}(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase1 * A_{col1} + ase2 * A_{col2} + ase3 * A_{web}) / A_{sec} = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$ase3 = 0$ (grid does not provide confinement)
 $psh_{min} = \text{Min}(psh_x, psh_y) = 0.0010472$

Expression ((5.4d), TBDY) for psh_{min} has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x = ps1_x + ps2_x + ps3_x = 0.00439823$
 $ps1_x$ (column 1) = $(As1 * h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 * ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2_x$ (column 2) = $(As2 * h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 * ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3_x$ (web) = $(As3 * h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 * ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh_y = ps1_y + ps2_y + ps3_y = 0.0010472$
 $ps1_y$ (column 1) = $(As1 * h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2_y$ (column 2) = $(As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3_y$ (web) = $(As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

```

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00
fywe = 625.00
fce = 25.00
From ((5.A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 750.00
fy1 = 625.00
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463

```

$c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)

--->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $s_u(4.9) = 0.17396707$
 $\mu_u = M_{Rc}(4.14) = 3.0844E+008$
 $u = s_u(4.1) = 0.00018625$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of μ_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 0.00019204$
 $\mu_u = 3.9637E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $\alpha_{co}(5A.5, TBDY) = 0.002$
 Final value of α_{cu} : $\alpha_{cu} = \text{shear_factor} * \text{Max}(\alpha_{cu}, \alpha_{cc}) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\alpha_{cu} = 0.0035$
 $\alpha_{we}((5.4c), TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe}/f_{ce} + \text{Min}(\alpha_{fx}, \alpha_{fy}) = 0.00$
 where $\alpha_f = \alpha_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\alpha_{fx} = 0.00$
 $\alpha_{af} = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $f_{fe} = 757.2164$

$\alpha_{fy} = 0.00$
 $\alpha_{af} = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00067733$
 $bw = 3000.00$
 effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$
 Effective FRP thickness, $t_f = N L * t * \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{,f} = 0.015$
 $\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web})/A_{sec} = 0.00$
 $\alpha_{se1} = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$

$ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$

```

fy2 = 625.00
su2 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lo,min = lb/lb,min = 1.00
    su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu2_nominal = 0.08,
    For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
    characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs2 = fs = 625.00
    with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lo,min = lb/ld = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 625.00
    with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
    c = confinement factor = 1.00246
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
    2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
    v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.19888934
Mu = MRc (4.14) = 3.9637E+008
u = su (4.1) = 0.00019204

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 0.00018625
Mu = 3.0844E+008

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\alpha (5A.5, \text{TB DY}) = 0.002$$

$$\text{Final value of } \alpha: \alpha = \text{shear_factor} * \text{Max}(\alpha_c, \alpha_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TB DY: } \alpha_c = 0.0035$$

$$\alpha_{we} ((5.4c), \text{TB DY}) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(\alpha_{fx}, \alpha_{fy}) = 0.00$$

where $\alpha_{fx} = \alpha_{se} * \alpha_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\alpha_{fx} = 0.00$$

$$\alpha_{se} = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \alpha_{pf} = 2t_f/b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\alpha_{fy} = 0.00$$

$$\alpha_{se} = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \alpha_{pf} = 2t_f/b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(\beta_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$\alpha_{u,f} = 0.015$$

$$\alpha_{se} ((5.4d), \text{TB DY}) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$\text{sh}_1 = 100.00$$

$$\text{bo}_1 = 190.00$$

$$\text{ho}_1 = 540.00$$

$$\text{bi2}_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$\text{sh}_2 = 100.00$$

$$\text{bo}_2 = 190.00$$

$$\text{ho}_2 = 540.00$$

$$\text{bi2}_2 = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\text{psh}_{\min} = \text{Min}(\text{psh}_x, \text{psh}_y) = 0.0010472$$

Expression ((5.4d), TB DY) for psh_{\min} has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\text{psh}_x = \text{ps1}_x + \text{ps2}_x + \text{ps3}_x = 0.00439823$$

$$\text{ps1}_x \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{\text{stir1}} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\text{ps2}_x \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{\text{stir2}} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$\text{ps3}_x \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{\text{stir3}} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$$\text{psh}_y = \text{ps1}_y + \text{ps2}_y + \text{ps3}_y = 0.0010472$$

$$\text{ps1}_y \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.0005236$$

$h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

 $Asec = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 625.00$

$fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 * esuv_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

```

with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

```

```

--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
u = 0.00019204
Mu = 3.9637E+008

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

```

```

fx = 0.00
af = 0.00
b = 250.00
h = 3000.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.008128
bw = 250.00
effective stress from (A.35), ffe = 757.2164

```

```

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00

```

effective stress from (A.35), $f_{f,e} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,e} = 0.015$

$a_{se}((5.4d), TBDY) = (a_{se1} \cdot A_{col1} + a_{se2} \cdot A_{col2} + a_{se3} \cdot A_{web}) / A_{sec} = 0.00$

$a_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$a_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$a_{se3} = 0$ (grid does not provide confinement)

$p_{sh,min} = \min(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$

$p_{s1,x}$ (column 1) = $(A_{s1} \cdot h_1 / s_1) / A_c = 0.00125664$

$h_1 = 600.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,x}$ (column 2) = $(A_{s2} \cdot h_2 / s_2) / A_c = 0.00125664$

$h_2 = 600.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,x}$ (web) = $(A_{s3} \cdot h_3 / s_3) / A_c = 0.00188496$

$h_3 = 1800.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 0.00$

No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$

$p_{s1,y}$ (column 1) = $(A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,y}$ (column 2) = $(A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,y}$ (web) = $(A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 157.0796$

No stirrups, $n_{s3} = 0.00$

$A_{sec} = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$f_{ywe} = 625.00$

$f_{ce} = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

c = confinement factor = 1.00246

$y_1 = 0.0025$

$sh_1 = 0.008$

$f_{t1} = 750.00$

$f_{y1} = 625.00$

$su_1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

```

lo/lou,min = lb/d = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
---->
v < vs,c - RHS eq.(4.5) is satisfied
---->
su (4.9) = 0.19888934
Mu = MRc (4.14) = 3.9637E+008
u = su (4.1) = 0.00019204

```

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2338\text{E}+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338\text{E}+006$
From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513\text{E}-012$

$V_u = 1.1832914\text{E}-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17, 10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i ,
as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \alpha)|, |V_f(-45, \alpha)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929\text{E}+006$

$b_w = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f'_c = 25.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $\mu_u = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $N_u = 27514.027$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 0.00$ is calculated for web, with:
 $d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$
 V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)
 $2(1-s/d) = 0.00$
 V_f ((11-3)-(11.4), ACI 440) = 109599.773
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 208.00
 f_{fe} ((11-5), ACI 440) = 259.312
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

 End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At local axis: 2

 Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
 At local axis: 2
 Integration Section: (a)
 Section Type: rcrcws

Constant Properties

 Knowledge Factor, $\phi = 0.85$
 According to 10.7.2.3, ASCE 41-17, shear walls with
 transverse reinforcement percentage, $\rho_n < 0.0015$
 are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17
 $\rho_n = 0.0010472$

 with $\rho_n = \rho_{ps1} + \rho_{ps2} + \rho_{ps3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 3

(pseudo-col.1 $ps1 = As1*b1/s1 = (As1*h1/s1) / Ac = 0.0005236$
 $h1 = 250.00$
 $s1 = 100.00$
total area of hoops perpendicular to shear axis, $As1 = 157.0796$
(pseudo-col.2 $ps2 = As2*b2/s2 = (As2*h2/s2) / Ac = 0.0005236$
 $h2 = 250.00$
 $s2 = 100.00$
total area of hoops perpendicular to shear axis, $As2 = 157.0796$
(grid $ps3 = As3*b3/s3 = (As3*h3/s3) / Ac = 0.00$
 $h3 = 250.00$
 $s3 = 200.00$
total area of hoops perpendicular to shear axis, $As3 = 0.00$
total section area, $Ac = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $fc = fc_lower_bound = 25.00$
New material of Secondary Member: Steel Strength, $fs = fs_lower_bound = 500.00$
Concrete Elasticity, $Ec = 26999.444$
Steel Elasticity, $Es = 200000.00$
Total Height, $H_{tot} = 3000.00$
Edges Width, $Wedg = 250.00$
Edges Height, $Hedg = 600.00$
Web Width, $Wweb = 250.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($lb/ld > 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $ffu = 1055.00$
Tensile Modulus, $Ef = 64828.00$
Elongation, $efu = 0.01$
Number of directions, $NoDir = 1$
Fiber orientations, $bi: 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Axial Force, $F = -31849.263$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $As_{lt} = 0.00$
-Compression: $As_{lc} = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $As_{l,ten} = 2865.133$
-Compression: $As_{l,com} = 2865.133$
-Middle: $As_{l,mid} = 615.7522$
Mean Diameter of Tension Reinforcement, $DbL = 17.33333$

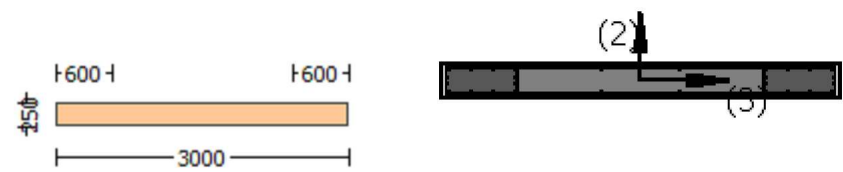
Considering wall controlled by Shear (shear control ratio > 1),
interstorey drift provided values are calculated
New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.004$
from table 10-20: $u = 0.004$
with:
- Condition i (shear wall and wall segments)
- $(As - As')*fy + P / (tw*lw*fc') = -0.1675285$
 $As = 0.00$

As' = 6346.017
fy = 500.00
P = 31849.263
tw = 250.00
lw = 3000.00
fc = 25.00

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
At local axis: 2
Integration Section: (a)

Calculation No. 3

wall W1, Floor 1
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)
Analysis: Uniform +X
Check: Shear capacity VRd
Edge: Start
Local Axis: (3)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1
At local axis: 3
Integration Section: (a)
Section Type: rcrrws

Constant Properties

Knowledge Factor, = 0.85
Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.
Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
New material of Secondary Member: Concrete Strength, fc = fc_lower_bound = 25.00
New material of Secondary Member: Steel Strength, fs = fs_lower_bound = 500.00

Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Total Height, $H_{tot} = 3000.00$
Edges Width, $W_{edg} = 250.00$
Edges Height, $H_{edg} = 600.00$
Web Width, $W_{web} = 250.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($l_o/l_{ou,min} = l_b/l_d \geq 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $f_{fu} = 1055.00$
Tensile Modulus, $E_f = 64828.00$
Elongation, $e_{fu} = 0.01$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-
Bending Moment, $M_a = 7.4461E+007$
Shear Force, $V_a = -25262.081$
EDGE -B-
Bending Moment, $M_b = 1.3414E+006$
Shear Force, $V_b = 25262.081$
BOTH EDGES
Axial Force, $F = -31849.263$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{sl,t} = 0.00$
-Compression: $A_{sl,c} = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 2865.133$
-Compression: $A_{sl,com} = 2865.133$
-Middle: $A_{sl,mid} = 615.7522$
Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.33333$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 2.4900E+006$
From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c' \cdot h \cdot d = 2.4900E+006$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 782300.325$
 $\mu_u/V_u - l_w/2 = 1447.552 > 0$
= 1 (normal-weight concrete)
 $f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $l_w = 3000.00$
 $\mu_u = 7.4461E+007$
 $V_u = 25262.081$
 $N_u = 31849.263$
From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.3195E+006$
 $V_{s1} = 376991.118$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 376991.118$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 565486.678$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_f ((11-3)-(11.4), \text{ACI 440}) = 1.5581\text{E}+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

$f_{fe} ((11-5), \text{ACI 440}) = 259.312$

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929\text{E}+006$

$b_w = 250.00$

End Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 4

wall W1, Floor 1

Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (θ_u)

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcwrs

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou}, \min \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $NoDir = 1$

Fiber orientations, $bi: 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = -3.7865323E-029$

EDGE -B-

Shear Force, $V_b = 3.7865323E-029$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2865.133$

-Compression: $As_{c,com} = 2865.133$

-Middle: $As_{mid} = 0.00$

(According to 10.7.2.3 As_{mid} is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 1.77991$

Member Controlled by Shear ($V_e/V_r > 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$ with

$M_{pr1} = \text{Max}(Mu_{1+}, Mu_{1-}) = 7.6379E+009$

$Mu_{1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+}, Mu_{2-}) = 7.6379E+009$

$Mu_{2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 1.1557999E-005$

$M_u = 6.9435E+009$

with full section properties:

$b = 250.00$

$d = 2957.00$

$d' = 43.00$

$v = 0.00112784$

$N = 27514.027$

$f_c = 33.00$

ϕ_c (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.0035$

ϕ_{we} ((5.4c), TBDY) = $a s_e * \phi_{sh,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.00$

where $\phi = a f * p f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\phi_x = 0.00$

$a f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $p f = 2 t f / b w = 0.008128$

$b w = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$\phi_y = 0.00$

$a f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $p f = 2 t f / b w = 0.00067733$

$b w = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase_1 \cdot A_{col1} + ase_2 \cdot A_{col2} + ase_3 \cdot A_{web}) / A_{sec} = 0.00$
 $ase_1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase_2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase_3 = 0$ (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $f_{ywe} = 694.45$
 $f_{ce} = 33.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c =$ confinement factor = 1.00246
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lo,min = lb/ld = 1.00$

```

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 694.45
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.00

```

and confined core properties:

```

b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.00

```

Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

```

--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.06369787
Mu = MRc (4.14) = 6.9435E+009
u = su (4.1) = 1.1557999E-005

```

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

Calculation of Mu1-

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.1645191E-005$$

$$M_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\phi_c \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.0035$$

$$\phi_{we} \text{ ((5.4c), TBDY)} = a_{se} * \phi_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = a_f * \phi_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$a_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{sh,min} = \text{Min}(\phi_{sh,x}, \phi_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{sh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$h_2 = 600.00$
 $As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,x} (web) = (As_3 * h_3 / s_3) / Ac = 0.00188496$
 $h_3 = 1800.00$
 $As_3 = Astir_3 * ns_3 = 0.00$
 No stirups, $ns_3 = 2.00$

$psh,y = ps_1,y + ps_2,y + ps_3,y = 0.0010472$
 $ps_{1,y} (column\ 1) = (As_1 * h_1 / s_1) / Ac = 0.0005236$
 $h_1 = 250.00$
 $As_1 = Astir_1 * ns_1 = 157.0796$
 No stirups, $ns_1 = 2.00$
 $ps_{2,y} (column\ 2) = (As_2 * h_2 / s_2) / Ac = 0.0005236$
 $h_2 = 250.00$
 $As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,y} (web) = (As_3 * h_3 / s_3) / Ac = 0.00$
 $h_3 = 250.00$
 $As_3 = Astir_3 * ns_3 = 157.0796$
 No stirups, $ns_3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fy_{we} = 694.45$
 $f_{ce} = 33.00$

From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 833.34$
 $fy_1 = 694.45$
 $su_1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su_1 = 0.4 * esu_{1,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = fs = 694.45$

with $Es_1 = Es = 200000.00$

$y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 833.34$
 $fy_2 = 694.45$
 $su_2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su_2 = 0.4 * esu_{2,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,

For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = fs = 694.45$

with $Es_2 = Es = 200000.00$

$y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v , sh_v , ft_v , fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1 , sh_1 , ft_1 , fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.0815606$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.0815606$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.01752837$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.10841651$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.10841651$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.02330004$

Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.07070831$
 $Mu = MRc (4.14) = 7.6379E+009$
 $u = su (4.1) = 1.1645191E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 1.1557999E-005$
 $Mu = 6.9435E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $we ((5.4c), TBDY) = ase * sh_{min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.00$
 where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$

From EC8 A4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
effective stress from (A.35), $ff,e = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff,e = 958.8339$

$R = 40.00$
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$ase3 = 0$ (grid does not provide confinement)
 $psh,min = Min(psh,x, psh,y) = 0.0010472$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3*ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $f_{ywe} = 694.45$
 $f_{ce} = 33.00$

From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 1.00$
 $su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/lb,min = 1.00$
 $su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou,min = lb/ld = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.06369787$$

$$M_u = M_{Rc}(4.14) = 6.9435E+009$$

$$u = s_u(4.1) = 1.1557999E-005$$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of M_{u2} -

Calculation of ultimate curvature κ_u according to 4.1, Biskinis/Fardis 2013:

$$\kappa_u = 1.1645191E-005$$

$$M_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\alpha_{co}(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \kappa_u: \kappa_u^* = \text{shear_factor} * \text{Max}(\kappa_u, \kappa_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \kappa_u = 0.0035$$

$$\omega_{se}((5.4c), TBDY) = \alpha_{se} * \text{sh}_{\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = \alpha_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$f_y = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,y (web) = (As3*h3/s_3)/Ac = 0.00
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirrups, ns3 = 0.00

Asec = 750000.00

s_1 = 100.00

s_2 = 100.00

s_3 = 200.00

fywe = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463

c = confinement factor = 1.00246

y1 = 0.0025

sh1 = 0.008

ft1 = 833.34

fy1 = 694.45

su1 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.

with fs1 = fs = 694.45

with Es1 = Es = 200000.00

y2 = 0.0025

sh2 = 0.008

ft2 = 833.34

fy2 = 694.45

su2 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $es_{u2_nominal} = 0.08$,
 For calculation of $es_{u2_nominal}$ and y_2 , sh_2, ft_2, fy_2 , it is considered
 characteristic value $fs_{y2} = fs_2/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 694.45$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $s_{uv} = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/d = 1.00$
 $s_{uv} = 0.4 \cdot es_{uv_nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $es_{uv_nominal} = 0.08$,
 considering characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY
 For calculation of $es_{uv_nominal}$ and y_v , sh_v, ft_v, fy_v , it is considered
 characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_v = fs = 694.45$
 with $Es_v = Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs_1 / fc) = 0.0815606$
 $2 = Asl, com / (b \cdot d) \cdot (fs_2 / fc) = 0.0815606$
 $v = Asl, mid / (b \cdot d) \cdot (fs_v / fc) = 0.01752837$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs_1 / fc) = 0.10841651$
 $2 = Asl, com / (b \cdot d) \cdot (fs_2 / fc) = 0.10841651$
 $v = Asl, mid / (b \cdot d) \cdot (fs_v / fc) = 0.02330004$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.07070831$
 $Mu = MRc (4.14) = 7.6379E+009$
 $u = su (4.1) = 1.1645191E-005$

 Calculation of ratio lb/d

Adequate Lap Length: $lb/d \geq 1$

 Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot fc' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$
 $Mu / Vu - lw / 2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $fc' = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$

lw = 3000.00

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661\text{E}+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

d = 1440.00

$A_v = 157079.633$

s = 200.00

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581\text{E}+006$

f = 0.95, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897\text{E}+006$

bw = 250.00

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 * f_c'^{0.5} * h * d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f * V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

h = 250.00

d = 2400.00

lw = 3000.00

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661\text{E}+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

d = 480.00

$A_v = 157079.633$

s = 100.00

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

d = 480.00

$A_v = 157079.633$

$s = 100.00$
 $f_y = 555.56$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 628323.557$ is calculated for web, with:
 $d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 555.56$
 V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_f ((11-3)-(11.4), \text{ACI 440}) = 1.5581\text{E}+006$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = N_L * t / \text{NoDir} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00
 $f_{fe} ((11-5), \text{ACI 440}) = 259.312$
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 2.2897\text{E}+006$
 $b_w = 250.00$

 End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At local axis: 3

 Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At Shear local axis: 2
 (Bending local axis: 3)
 Section Type: rcrcws

Constant Properties

 Knowledge Factor, $\phi = 0.85$
 Mean strength values are used for both shear and moment calculations.
 Consequently:
 New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$
 New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 #####
 Note: Especially for the calculation of moment strengths,
 the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
 New material: Steel Strength, $f_s = 1.25 * f_{sm} = 625.00$
 #####
 Total Height, $H_{tot} = 3000.00$
 Edges Width, $W_{edg} = 250.00$
 Edges Height, $H_{edg} = 600.00$
 Web Width, $W_{web} = 250.00$
 Cover Thickness, $c = 25.00$
 Mean Confinement Factor overall section = 1.00246
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou, \min} \geq 1$)
 FRP Wrapping Data
 Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $NoDir = 1$

Fiber orientations, $b_i = 0.00^\circ$

Number of layers, $NL = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 1.1832914E-030$

EDGE -B-

Shear Force, $V_b = -1.1832914E-030$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2368.761$

-Compression: $As_{l,com} = 2368.761$

-Middle: $As_{l,mid} = 0.00$

(According to 10.7.2.3 $As_{l,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 0.21417884$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$

with

$M_{pr1} = \text{Max}(\mu_{u1+}, \mu_{u1-}) = 3.9637E+008$

$\mu_{u1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$\mu_{u1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(\mu_{u2+}, \mu_{u2-}) = 3.9637E+008$

$\mu_{u2+} = 3.0844E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$\mu_{u2-} = 3.9637E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 0.00018625$

$\mu_u = 3.0844E+008$

with full section properties:

$b = 3000.00$

$d = 208.00$

$d' = 42.00$

$v = 0.00176372$

$N = 27514.027$

$f_c = 25.00$

ϕ_o (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_o) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.0035$

we ((5.4c), TBDY) = $ase \cdot sh_{min} \cdot fy_{we}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$
where $f = af \cdot pf \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
effective stress from (A.35), $ff_e = 757.2164$

$f_y = 0.00$
 $af = 0.00$
 $b = 3000.00$
 $h = 250.00$
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff_e = 954.4864$

$R = 40.00$
Effective FRP thickness, $tf = NL \cdot t \cdot \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase \text{ ((5.4d), TBDY)} = (ase_1 \cdot A_{col1} + ase_2 \cdot A_{col2} + ase_3 \cdot A_{web})/A_{sec} = 0.00$
 $ase_1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase_2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$ase_3 = 0$ (grid does not provide confinement)
 $psh_{min} = \text{Min}(psh_x, psh_y) = 0.0010472$

Expression ((5.4d), TBDY) for psh_{min} has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh_x = ps1_x + ps2_x + ps3_x = 0.00439823$
 $ps1_x \text{ (column 1)} = (As1 \cdot h1/s_1)/Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2_x \text{ (column 2)} = (As2 \cdot h2/s_2)/Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3_x \text{ (web)} = (As3 \cdot h3/s_3)/Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh_y = ps1_y + ps2_y + ps3_y = 0.0010472$
 $ps1_y \text{ (column 1)} = (As1 \cdot h1/s_1)/Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2_y \text{ (column 2)} = (As2 \cdot h2/s_2)/Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3_y \text{ (web)} = (As3 \cdot h3/s_3)/Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

```

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00
fywe = 625.00
fce = 25.00
From ((5.A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 750.00
fy1 = 625.00
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463

```

$c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)

--->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $s_u(4.9) = 0.17396707$
 $\mu_u = M_{Rc}(4.14) = 3.0844E+008$
 $u = s_u(4.1) = 0.00018625$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of μ_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 0.00019204$
 $\mu_u = 3.9637E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $\alpha(5A.5, TBDY) = 0.002$

Final value of α_u : $\alpha_u = \text{shear_factor} * \text{Max}(\alpha_u, \alpha_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\alpha_u = 0.0035$

$\alpha_{we}((5.4c), TBDY) = \alpha_{se} * \text{sh}_{,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = \alpha_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{f,e} = 757.2164$

$f_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{f,e} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = N L * t * \text{Cos}(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{,f} = 0.015$

$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web})/A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_{_1} = 100.00$

$bo_{_1} = 190.00$

$ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lo_{u,min} = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$

```

fy2 = 625.00
su2 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu2_nominal = 0.08,
    For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
    characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs2 = fs = 625.00
    with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/ld = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 625.00
    with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
    c = confinement factor = 1.00246
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
    2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
    v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.19888934
Mu = MRc (4.14) = 3.9637E+008
u = su (4.1) = 0.00019204

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 0.00018625
Mu = 3.0844E+008

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\alpha (5A.5, \text{TB DY}) = 0.002$$

$$\text{Final value of } \alpha: \alpha = \text{shear_factor} * \text{Max}(\alpha_c, \alpha_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TB DY: } \alpha_c = 0.0035$$

$$\alpha_{ve} ((5.4c), \text{TB DY}) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(\alpha_{fx}, \alpha_{fy}) = 0.00$$

where $\alpha_{fx} = \alpha_{se} * \alpha_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\alpha_{fx} = 0.00$$

$$\alpha_{fe} = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \alpha_{pf} = 2t_f / b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\alpha_{fy} = 0.00$$

$$\alpha_{fe} = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \alpha_{pf} = 2t_f / b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(\beta_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$\alpha_{u,f} = 0.015$$

$$\alpha_{se} ((5.4d), \text{TB DY}) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$\text{sh}_1 = 100.00$$

$$\text{bo}_1 = 190.00$$

$$\text{ho}_1 = 540.00$$

$$\text{bi2}_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$\text{sh}_2 = 100.00$$

$$\text{bo}_2 = 190.00$$

$$\text{ho}_2 = 540.00$$

$$\text{bi2}_2 = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\text{psh}_{\min} = \text{Min}(\text{psh}_x, \text{psh}_y) = 0.0010472$$

Expression ((5.4d), TB DY) for psh_{\min} has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\text{psh}_x = \text{ps1}_x + \text{ps2}_x + \text{ps3}_x = 0.00439823$$

$$\text{ps1}_x \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\text{ps2}_x \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$\text{ps3}_x \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{stir3} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$$\text{psh}_y = \text{ps1}_y + \text{ps2}_y + \text{ps3}_y = 0.0010472$$

$$\text{ps1}_y \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.0005236$$

$h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

 $Asec = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 625.00$

$fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 * esuv_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.


```

with fsv = fs = 625.00
with Es = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

```

```

--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
u = 0.00019204
Mu = 3.9637E+008

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

```

```

fx = 0.00
af = 0.00
b = 250.00
h = 3000.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.008128
bw = 250.00
effective stress from (A.35), ffe = 757.2164

```

```

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00

```

effective stress from (A.35), $f_{f,e} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_f = 0.015$

$a_{se}((5.4d), TBDY) = (a_{se1} \cdot A_{col1} + a_{se2} \cdot A_{col2} + a_{se3} \cdot A_{web}) / A_{sec} = 0.00$

$a_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$a_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$a_{se3} = 0$ (grid does not provide confinement)

$p_{sh,min} = \min(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$

$p_{s1,x}$ (column 1) = $(A_{s1} \cdot h_1 / s_1) / A_c = 0.00125664$

$h_1 = 600.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,x}$ (column 2) = $(A_{s2} \cdot h_2 / s_2) / A_c = 0.00125664$

$h_2 = 600.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,x}$ (web) = $(A_{s3} \cdot h_3 / s_3) / A_c = 0.00188496$

$h_3 = 1800.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 0.00$

No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$

$p_{s1,y}$ (column 1) = $(A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,y}$ (column 2) = $(A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,y}$ (web) = $(A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 157.0796$

No stirrups, $n_{s3} = 0.00$

$A_{sec} = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$f_{ywe} = 625.00$

$f_{ce} = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

c = confinement factor = 1.00246

$y_1 = 0.0025$

$sh_1 = 0.008$

$f_{t1} = 750.00$

$f_{y1} = 625.00$

$su_1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

```

lo/lou,min = lb/d = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
---->
v < vs,c - RHS eq.(4.5) is satisfied
---->
su (4.9) = 0.19888934
Mu = MRc (4.14) = 3.9637E+008
u = su (4.1) = 0.00019204

```

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2338\text{E}+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) 'Vw' is replaced by 'Vw + f * Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513\text{E}-012$

$V_u = 1.1832914\text{E}-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17, 10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929\text{E}+006$

$b_w = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) 'Vw' is replaced by 'Vw + f * Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f'_c = 25.00$, but $f'_c^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $\mu_u = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $N_u = 27514.027$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 0.00$ is calculated for web, with:
 $d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$
 V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)
 $2(1-s/d) = 0.00$
 V_f ((11-3)-(11.4), ACI 440) = 109599.773
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 208.00
 f_{fe} ((11-5), ACI 440) = 259.312
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

 End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At local axis: 2

 Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
 At local axis: 3
 Integration Section: (a)
 Section Type: rcrws

Constant Properties

 Knowledge Factor, $\gamma = 0.85$
 According to 10.7.2.3, ASCE 41-17, shear walls with
 transverse reinforcement percentage, $\rho_n < 0.0015$
 are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17
 $\rho_n = 0.0010472$

 with $\rho_n = \rho_{ps1} + \rho_{ps2} + \rho_{ps3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 2

(pseudo-col.1 $ps1 = As1*b1/s1 = (As1*h1/s1) / Ac = 0.0005236$
 $h1 = 250.00$
 $s1 = 100.00$
total area of hoops perpendicular to shear axis, $As1 = 157.0796$
(pseudo-col.2 $ps2 = As2*b2/s2 = (As2*h2/s2) / Ac = 0.0005236$
 $h2 = 250.00$
 $s2 = 100.00$
total area of hoops perpendicular to shear axis, $As2 = 157.0796$
(grid $ps3 = As3*b3/s3 = (As3*h3/s3) / Ac = 0.00$
 $h3 = 250.00$
 $s3 = 200.00$
total area of hoops perpendicular to shear axis, $As3 = 0.00$
total section area, $Ac = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $fc = fc_lower_bound = 25.00$
New material of Secondary Member: Steel Strength, $fs = fs_lower_bound = 500.00$
Concrete Elasticity, $Ec = 26999.444$
Steel Elasticity, $Es = 200000.00$
Total Height, $H_{tot} = 3000.00$
Edges Width, $Wedg = 250.00$
Edges Height, $Hedg = 600.00$
Web Width, $Wweb = 250.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($lb/ld > 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $ffu = 1055.00$
Tensile Modulus, $Ef = 64828.00$
Elongation, $efu = 0.01$
Number of directions, $NoDir = 1$
Fiber orientations, $bi: 0.00^\circ$
Number of layers, $NL = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 4.1575044E-011$
Shear Force, $V2 = 8.1671580E-014$
Shear Force, $V3 = -25262.081$
Axial Force, $F = -31849.263$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $Aslt = 0.00$
-Compression: $Aslc = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $Asl,ten = 2368.761$
-Compression: $Asl,com = 2368.761$
-Middle: $Asl,mid = 1608.495$
Mean Diameter of Tension Reinforcement, $DbL = 17.20$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.00383542$
 $u = y + p = 0.00383542$

- Calculation of y -

$y = (M_y \cdot I_p) / (E I)_{\text{Eff}} = 0.00183542 \text{ ((10-5), ASCE 41-17)}$
 $M_y = 2.8230\text{E}+008$
 $(E I)_{\text{Eff}} = 0.35 \cdot E_c \cdot I \text{ (table 10-5)}$
 $E_c \cdot I = 1.0547\text{E}+014$
 $I_p = 0.5 \cdot d = 0.5 \cdot (0.8 \cdot h) = 240.00$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{\text{ten}}, y_{\text{com}})$
 $y_{\text{ten}} = 1.5746739\text{E}-005$
 with $f_y = 500.00$
 $d = 208.00$
 $y = 0.23671621$
 $A = 0.01027198$
 $B = 0.0062138$
 with $p_t = 0.00379609$
 $p_c = 0.00379609$
 $p_v = 0.00257772$
 $N = 31849.263$
 $b = 3000.00$
 $" = 0.20192308$
 $y_{\text{comp}} = 3.3996131\text{E}-005$
 with $f_c^* (12.3, \text{ACI 440}) = 25.002$
 $f_c = 25.00$
 $f_l = 0.17503396$
 $b = 3000.00$
 $h = 250.00$
 $A_g = 750000.00$
 From (12.9), ACI 440: $k_a = 0.00364754$
 $g = p_t + p_c + p_v = 0.0101699$
 $r_c = 40.00$
 $A_e/A_c = 0.52524587$
 Effective FRP thickness, $t_f = N L \cdot t \cdot \cos(b_1) = 1.016$
 effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$
 $f_u = 0.01$
 $E_f = 64828.00$
 $E_c = 26999.444$
 $y = 0.23572173$
 $A = 0.01001679$
 $B = 0.00611172$
 with $E_s = 200000.00$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

- Calculation of p -

Considering wall controlled by flexure (shear control ratio ≤ 1),
 from table 10-19: $p = 0.002$
 with:

- Condition i (shear wall and wall segments)
 - $(A_s - A_s') \cdot f_y + P / (t_w \cdot l_w \cdot f_c') = -0.1675285$
 $A_s = 0.00$
 $A_s' = 6346.017$
 $f_y = 500.00$
 $P = 31849.263$
 $t_w = 3000.00$
 $l_w = 250.00$
 $f_c = 25.00$

- $V/(t_w \cdot l_w \cdot f_c^{0.5}) = 2.6227864E-019$, NOTE: units in lb & in
 - Confined Boundary: No
 Boundary hoops spacing does not exceed $8d_b$ ($s1 < 8 \cdot d_b$ and $s2 < 8 \cdot d_b$)
 Boundary Trans. Reinf. does not exceed 50% of ACI 318 provision ($V_{w1} + V_{w2} > 0.50 \cdot (V - V_c - V_{w3})$)
 With
 Boundary Element 1:
 $V_{w1} = 157079.633$
 $s1 = 100.00$
 Boundary Element 2:
 $V_{w2} = 157079.633$
 $s2 = 100.00$
 Grid Shear Force, $V_{w3} = 0.00$
 Concrete Shear Force, $V_c = 348603.471$
 (The variables above have already been given in Shear control ratio calculation)
 Mean diameter of all bars, $d_b = 17.33333$
 Design Shear Force, $V = 8.1671580E-014$

 End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
 At local axis: 3
 Integration Section: (a)

Calculation No. 5

wall W1, Floor 1
 Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)
 Analysis: Uniform +X
 Check: Shear capacity V_{Rd}
 Edge: End
 Local Axis: (2)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1
 At local axis: 2
 Integration Section: (d)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\phi = 0.85$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{o,min} = l_b/l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-

Bending Moment, $M_a = 4.1575044E-011$

Shear Force, $V_a = 8.1671580E-014$

EDGE -B-

Bending Moment, $M_b = 1.9900347E-010$

Shear Force, $V_b = -8.1671580E-014$

BOTH EDGES

Axial Force, $F = -31849.263$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{sl,t} = 0.00$

-Compression: $A_{sl,c} = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $A_{sl,ten} = 2368.761$

-Compression: $A_{sl,com} = 2368.761$

-Middle: $A_{sl,mid} = 1608.495$

Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.20$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 601274.794$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d = 601274.794$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 182995.745$

$M_u/V_u - l_w/2 = 2311.631 > 0$

$\lambda = 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $M_u = 1.9900347E-010$
 $V_u = 8.1671580E-014$
 $N_u = 31849.263$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:
 $d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 0.00$ is calculated for web, with:
 $d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$
 V_{s3} has been multiplied by 2(1-s/d) ($s > d/2$, according to ASCE 41-17,10.3.4)
 $2(1-s/d) = 0.00$
 $V_f ((11-3)-(11.4), \text{ACI 440}) = 109599.773$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta = 45^\circ$
 $V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a)|)$, with:
 total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 208.00
 $f_{fe} ((11-5), \text{ACI 440}) = 259.312$
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

End Of Calculation of Shear Capacity for element: wall W1 of floor 1
 At local axis: 2
 Integration Section: (d)

Calculation No. 6

wall W1, Floor 1

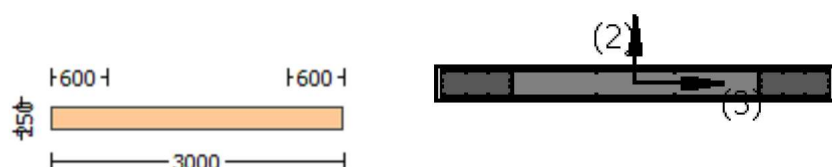
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (μ)

Edge: End

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcrrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou,min} \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, NoDir = 1
Fiber orientations, bi: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 3
EDGE -A-
Shear Force, Va = -3.7865323E-029
EDGE -B-
Shear Force, Vb = 3.7865323E-029
BOTH EDGES
Axial Force, F = -27514.027
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: Aslt = 0.00
-Compression: Aslc = 6346.017
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten = 2865.133
-Compression: Asl,com = 2865.133
-Middle: Asl,mid = 0.00
(According to 10.7.2.3 Asl,mid is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 1.77991$
Member Controlled by Shear ($V_e/V_r > 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$
with
 $M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 7.6379E+009$
 $M_{u1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $M_{u1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 7.6379E+009$
 $M_{u2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
which is defined for the the static loading combination
 $M_{u2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:
 $\phi_u = 1.1557999E-005$
 $M_u = 6.9435E+009$

with full section properties:

b = 250.00
d = 2957.00
d' = 43.00
 $\nu = 0.00112784$
N = 27514.027
 $f_c = 33.00$
 $\phi_{co} (5A.5, TBDY) = 0.002$
Final value of ϕ_{cu} : $\phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.0035$
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: $\phi_{cu} = 0.0035$
 $\phi_{we} ((5.4c), TBDY) = a_{se} * \phi_{sh, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$
where $\phi_f = a_f * \phi_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
 $\phi_{fx} = 0.00$
 $\phi_{af} = 0.00$

b = 250.00
h = 3000.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.008128$
bw = 250.00
effective stress from (A.35), $ff,e = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $ff,e = 958.8339$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00
ase3 = 0 (grid does not provide confinement)
 $psh,min = Min(psh,x, psh,y) = 0.0010472$
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
 $As1 = Astir1*ns1 = 157.0796$
No stirups, ns1 = 2.00
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
 $As2 = Astir2*ns2 = 157.0796$
No stirups, ns2 = 2.00
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
 $As3 = Astir3*ns3 = 0.00$
No stirups, ns3 = 2.00

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
 $As1 = Astir1*ns1 = 157.0796$
No stirups, ns1 = 2.00
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
 $As2 = Astir2*ns2 = 157.0796$
No stirups, ns2 = 2.00
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
 $As3 = Astir3*ns3 = 157.0796$
No stirups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

```

fywe = 694.45
fce = 33.00
From ((5A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 694.45
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfinedsd full section - Steel rupture

```

satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$s_u(4.9) = 0.06369787$

$M_u = M_{Rc}(4.14) = 6.9435E+009$

$u = s_u(4.1) = 1.1557999E-005$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of M_{u1} -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$

$M_u = 7.6379E+009$

with full section properties:

$b = 250.00$

$d = 2957.00$

$d' = 43.00$

$v = 0.00112784$

$N = 27514.027$

$f_c = 33.00$

$\alpha(5A.5, TBDY) = 0.002$

Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\alpha = 0.0035$

$\alpha_e((5.4c), TBDY) = \alpha * \text{sh_min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = \alpha * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \text{Cos}(b1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi_{2,1} = 655400.00$

$\alpha_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 694.45$
 $fce = 33.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 1.00$
 $su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$


```

lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
'satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.07070831
Mu = MRc (4.14) = 7.6379E+009
u = su (4.1) = 1.1645191E-005

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.1557999E-005
Mu = 6.9435E+009

with full section properties:

b = 250.00
d = 2957.00
d' = 43.00
v = 0.00112784

$N = 27514.027$
 $f_c = 33.00$
 $\alpha (5A.5, TBDY) = 0.002$
 Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\alpha = 0.0035$
 $\alpha_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$
 where $f = \alpha * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $\alpha_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$
 $\alpha_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00067733$
 $bw = 3000.00$
 effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{f,f} = 0.015$
 $\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$
 $\alpha_{se1} = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $\alpha_{se2} = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $\alpha_{se3} = 0$ (grid does not provide confinement)
 $p_{sh,\min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$
 Expression ((5.4d), TBDY) for $p_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$
 $p_{s1,x}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.00125664$
 $h_1 = 600.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,x}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.00125664$
 $h_2 = 600.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $p_{s3,x}$ (web) = $(A_{s3} * h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} * n_{s3} = 0.00$
 No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$
 $p_{s1,y}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,y}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.0005236$
 $h_2 = 250.00$

$As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y (web) = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 694.45$
 $fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y2, sh2, ft2, fy2$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

yv, shv, ftv, fyv , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 694.45$

with $Esv = Es = 200000.00$

$1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0815606$

$2 = Asl,com / (b * d) * (fs2 / fc) = 0.0815606$

$v = Asl,mid / (b * d) * (fsv / fc) = 0.00$

and confined core properties:

$$b = 190.00$$

$$d = 2927.00$$

$$d' = 13.00$$

$$fcc(5A.2, TBDY) = 33.08129$$

$$cc(5A.5, TBDY) = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.10841651$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.10841651$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.06369787$$

$$\mu_u = M_{Rc}(4.14) = 6.9435E+009$$

$$u = s_u(4.1) = 1.1557999E-005$$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of μ_{u2} -

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.1645191E-005$$

$$\mu_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$c_o(5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.0035$$

$$w_e((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L^* t \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u, f = 0.015$$

$$ase((5.4d), TBDY) = (ase1 \cdot A_{col1} + ase2 \cdot A_{col2} + ase3 \cdot A_{web}) / A_{sec} = 0.00$$

$$ase1 = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$ase2 = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$ase3 = 0 \text{ (grid does not provide confinement)}$$

$$psh, min = \min(psh, x, psh, y) = 0.0010472$$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh, x = ps1, x + ps2, x + ps3, x = 0.00439823$$

$$ps1, x \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.00125664$$

$$h1 = 600.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2, x \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.00125664$$

$$h2 = 600.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3, x \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00188496$$

$$h3 = 1800.00$$

$$As3 = Astir3 \cdot ns3 = 0.00$$

$$\text{No stirrups, } ns3 = 2.00$$

$$psh, y = ps1, y + ps2, y + ps3, y = 0.0010472$$

$$ps1, y \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.0005236$$

$$h1 = 250.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2, y \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.0005236$$

$$h2 = 250.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3, y \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00$$

$$h3 = 250.00$$

$$As3 = Astir3 \cdot ns3 = 157.0796$$

$$\text{No stirrups, } ns3 = 0.00$$

$$A_{sec} = 750000.00$$

$$s_1 = 100.00$$

$$s_2 = 100.00$$

$$s_3 = 200.00$$

$$fy_{we} = 694.45$$

$$f_{ce} = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$y1 = 0.0025$$

$$sh1 = 0.008$$

$$ft1 = 833.34$$

$$fy1 = 694.45$$

$$su1 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$lo/lo_{u,min} = lb/ld = 1.00$$

$$su1 = 0.4 \cdot esu1_{nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1, ft1, fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/lb, min = 1.00$
 $su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/ld = 1.00$
 $suv = 0.4 \cdot esuv_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.0815606$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.0815606$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.01752837$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc \text{ (5A.2, TBDY)} = 33.08129$
 $cc \text{ (5A.5, TBDY)} = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.10841651$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.10841651$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.02330004$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

$v < vs, y2$ - LHS eq.(4.5) is satisfied

$su \text{ (4.9)} = 0.07070831$

$Mu = MRc \text{ (4.14)} = 7.6379E+009$

$u = su \text{ (4.1)} = 1.1645191E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Shear Strength $Vr = \text{Min}(Vr1, Vr2) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$\mu_u = 3.1013748E-010$

$V_u = 3.7865323E-029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i ,
as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$b_w = 250.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = NL * t / \text{NoDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$bw = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcrcws

Constant Properties

Knowledge Factor, $\phi = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 * f_{sm} = 625.00$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{\text{edg}} = 600.00$
 Web Width, $W_{\text{web}} = 250.00$
 Cover Thickness, $c = 25.00$
 Mean Confinement Factor overall section = 1.00246
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{o,u}, \min > = 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $\epsilon_{fu} = 0.01$
 Number of directions, $N_{\text{Dir}} = 1$
 Fiber orientations, $\theta_i = 0.00^\circ$
 Number of layers, $N_L = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
 EDGE -A-
 Shear Force, $V_a = 1.1832914\text{E-}030$
 EDGE -B-
 Shear Force, $V_b = -1.1832914\text{E-}030$
 BOTH EDGES
 Axial Force, $F = -27514.027$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $A_{s,t} = 0.00$
 -Compression: $A_{s,c} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $A_{s,t,ten} = 2368.761$
 -Compression: $A_{s,c,com} = 2368.761$
 -Middle: $A_{s,mid} = 0.00$
 (According to 10.7.2.3 $A_{s,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 0.21417884$
 Member Controlled by Flexure ($V_e/V_r < 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
 with
 $M_{pr1} = \text{Max}(\mu_{u1+}, \mu_{u1-}) = 3.9637\text{E}+008$
 $\mu_{u1+} = 3.0844\text{E}+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination
 $\mu_{u1-} = 3.9637\text{E}+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(\mu_{u2+}, \mu_{u2-}) = 3.9637\text{E}+008$
 $\mu_{u2+} = 3.0844\text{E}+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination
 $\mu_{u2-} = 3.9637\text{E}+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:
 $\phi_u = 0.00018625$

$$\mu = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\alpha (5A.5, \text{TB DY}) = 0.002$$

$$\text{Final value of } \mu: \mu^* = \text{shear_factor} * \text{Max}(\mu, \alpha) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TB DY: } \mu = 0.0035$$

$$\mu_e ((5.4c), \text{TB DY}) = \alpha * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = \alpha * \mu * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \mu_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$f_y = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \mu_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$\alpha_{se} ((5.4d), \text{TB DY}) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\mu_{sh,\min} = \text{Min}(\mu_{sh,x}, \mu_{sh,y}) = 0.0010472$$

Expression ((5.4d), TB DY) for $\mu_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\mu_{sh,x} = \mu_{s1,x} + \mu_{s2,x} + \mu_{s3,x} = 0.00439823$$

$$\mu_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\mu_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$\mu_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{stir3} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

Asec = 750000.00

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 625.00$

$fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y2, sh2, ft2, fy2$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 \cdot esuv_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = f_s = 625.00$
 with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.09490228$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.09490228$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.00$

and confined core properties:

$b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} \text{ (5A.2, TBDY)} = 25.06158$
 $cc \text{ (5A.5, TBDY)} = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$\mu_u \text{ (4.9)} = 0.17396707$
 $\mu_u = M_{Rc} \text{ (4.14)} = 3.0844E+008$
 $u = \mu_u \text{ (4.1)} = 0.00018625$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of μ_{u1} -

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$u = 0.00019204$
 $\mu_u = 3.9637E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $cc \text{ (5A.5, TBDY)} = 0.002$
 Final value of μ_u : $\mu_u^* = \text{shear_factor} \cdot \text{Max}(\mu_u, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\mu_u = 0.0035$
 $\mu_{ue} \text{ ((5.4c), TBDY)} = a_{se} \cdot sh_{min} \cdot f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$
 where $f = a_f \cdot p_f \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $a_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $f_{fe} = 757.2164$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$

From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff,e = 954.4864$

$R = 40.00$
Effective FRP thickness, $tf = NL*t*\cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{,f} = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3*ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c =$ confinement factor = 1.00246

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $su_1 = 0.4 * esu_{1,nominal} ((5.5), TBDY) = 0.032$
From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,
For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
characteristic value $fsy_1 = fs_1/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
with $fs_1 = fs = 625.00$
with $Es_1 = Es = 200000.00$
 $y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 750.00$
 $fy_2 = 625.00$
 $su_2 = 0.032$
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_{b,min} = 1.00$
 $su_2 = 0.4 * esu_{2,nominal} ((5.5), TBDY) = 0.032$
From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,
For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
with $fs_2 = fs = 625.00$
with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $suv = 0.032$
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
with $fsv = fs = 625.00$
with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b * d) * (fs_1/f_c) = 0.09490228$
 $2 = Asl_{com}/(b * d) * (fs_2/f_c) = 0.09490228$
 $v = Asl_{mid}/(b * d) * (fsv/f_c) = 0.06444293$
and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b * d) * (fs_1/f_c) = 0.11316025$
 $2 = Asl_{com}/(b * d) * (fs_2/f_c) = 0.11316025$
 $v = Asl_{mid}/(b * d) * (fsv/f_c) = 0.07684091$
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
--->
 $su (4.9) = 0.19888934$
 $Mu = MRc (4.14) = 3.9637E+008$
 $u = su (4.1) = 0.00019204$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of μ_{2+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 0.00018625$$

$$\mu_u = 3.0844 \times 10^{-8}$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\mu_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \mu_u: \mu_u = \text{shear_factor} * \text{Max}(\mu_o, \mu_o) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu_u = 0.0035$$

$$\mu_{ve} \text{ ((5.4c), TBDY)} = a_{se} * \mu_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\mu_{fx}, \mu_{fy}) = 0.00$$

where $\mu_f = a_f * \mu_{pf} * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\mu_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \mu_{pf} = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\mu_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \mu_{pf} = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$\mu_{u,f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$\mu_{sh,1} = 100.00$$

$$\mu_{bo,1} = 190.00$$

$$\mu_{ho,1} = 540.00$$

$$\mu_{bi2,1} = 655400.00$$

$$a_{se2} = 0.00$$

$$\mu_{sh,2} = 100.00$$

$$\mu_{bo,2} = 190.00$$

$$\mu_{ho,2} = 540.00$$

$$\mu_{bi2,2} = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\mu_{psh,min} = \text{Min}(\mu_{psh,x}, \mu_{psh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\mu_{psh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\mu_{psh,x} = \mu_{ps1,x} + \mu_{ps2,x} + \mu_{ps3,x} = 0.00439823$$

$$\mu_{ps1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,x \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,x \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 * ns3 = 0.00$
 No stirups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 * h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$


```

ftv = 750.00
fyv = 625.00
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lo,min = lb/ld = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 625.00
    with Esv = Es = 200000.00
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
    2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
    c = confinement factor = 1.00246
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
    2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
---->
v < vs,c - RHS eq.(4.5) is satisfied
---->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

```

u = 0.00019204
Mu = 3.9637E+008

```

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
fx = 0.00

```

af = 0.00
b = 250.00
h = 3000.00
From EC8 A.4.4.3(6), pf = $2tf/bw = 0.008128$
bw = 250.00
effective stress from (A.35), ff,e = 757.2164

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), pf = $2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), ff,e = 954.4864

R = 40.00
Effective FRP thickness, tf = $NL*t*\cos(b1) = 1.016$
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015

ase ((5.4d), TBDY) = $(ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$

ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00

ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,y (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00

$s_3 = 200.00$
 $f_{ywe} = 625.00$
 $f_{ce} = 25.00$
 From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 750.00$
 $fy_1 = 625.00$
 $su_1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/ld = 1.00$
 $su_1 = 0.4 * esu_1_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu_1_{\text{nominal}} = 0.08$,
 For calculation of esu_1_{nominal} and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_1 = fs = 625.00$
 with $Es_1 = Es = 200000.00$
 $y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 750.00$
 $fy_2 = 625.00$
 $su_2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/lb, \min = 1.00$
 $su_2 = 0.4 * esu_2_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu_2_{\text{nominal}} = 0.08$,
 For calculation of esu_2_{nominal} and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 625.00$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/ld = 1.00$
 $suv = 0.4 * esuv_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esuv_{\text{nominal}} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{\text{nominal}}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = A_{sl, \text{ten}} / (b * d) * (fs_1 / fc) = 0.09490228$
 $2 = A_{sl, \text{com}} / (b * d) * (fs_2 / fc) = 0.09490228$
 $v = A_{sl, \text{mid}} / (b * d) * (fsv / fc) = 0.06444293$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, \text{TBDY}) = 25.06158$
 $cc (5A.5, \text{TBDY}) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl, \text{ten}} / (b * d) * (fs_1 / fc) = 0.11316025$
 $2 = A_{sl, \text{com}} / (b * d) * (fs_2 / fc) = 0.11316025$
 $v = A_{sl, \text{mid}} / (b * d) * (fsv / fc) = 0.07684091$

Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

μ_u (4.9) = 0.19888934

$M_u = M_{Rc}$ (4.14) = 3.9637E+008

$u = \mu_u$ (4.1) = 0.00019204

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / \mu_u - l_w/2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513E-012$

$V_u = 1.1832914E-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s \leq d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s \leq d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b_1 + 90^\circ = 90.00$

$V_f = \min(|V_f(45, \theta)|, |V_f(-45, a)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$
 $\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c' \cdot 0.5 \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $\mu_u = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 2
Integration Section: (a)
Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$
According to 10.7.2.3, ASCE 41-17, shear walls with
transverse reinforcement percentage, $\rho_n < 0.0015$
are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17
 $\rho_n = 0.0010472$

with $\rho_n = \rho_{s1} + \rho_{s2} + \rho_{s3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 3
(pseudo-col.1 $\rho_{s1} = A_{s1} \cdot b_1 / s_1 = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$s_1 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s1} = 157.0796$

(pseudo-col.2 $\rho_{s2} = A_{s2} \cdot b_2 / s_2 = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$s_2 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s2} = 157.0796$

(grid $\rho_{s3} = A_{s3} \cdot b_3 / s_3 = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$s_3 = 200.00$

total area of hoops perpendicular to shear axis, $A_{s3} = 0.00$

total section area, $A_c = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_b / d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Axial Force, $F = -31849.263$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $A_{sc} = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $A_{st,ten} = 2865.133$

-Compression: $A_{st,com} = 2865.133$

-Middle: $A_{st,mid} = 615.7522$

Mean Diameter of Tension Reinforcement, $D_{bL} = 17.33333$

Considering wall controlled by Shear (shear control ratio > 1),
interstorey drift provided values are calculated

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.004$
from table 10-20: $u = 0.004$

with:

- Condition i (shear wall and wall segments)

- $(A_s - A_s') \cdot f_y + P / (t_w \cdot l_w \cdot f_c') = -0.1675285$

$A_s = 0.00$

$A_s' = 6346.017$

$f_y = 500.00$

$P = 31849.263$

$t_w = 250.00$

$l_w = 3000.00$

$f_c = 25.00$

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 7

wall W1, Floor 1

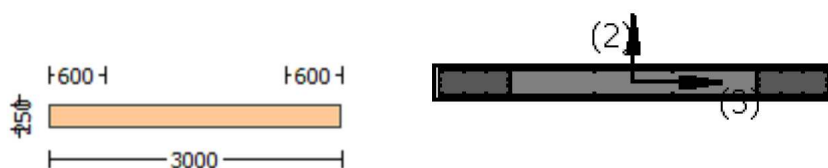
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1
At local axis: 3
Integration Section: (d)
Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$
Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.
Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17
Consequently:
New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$
New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Total Height, $H_{tot} = 3000.00$
Edges Width, $W_{edg} = 250.00$
Edges Height, $H_{edg} = 600.00$
Web Width, $W_{web} = 250.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($l_o/l_{ou,min} = l_b/l_d \geq 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $f_{fu} = 1055.00$
Tensile Modulus, $E_f = 64828.00$
Elongation, $\epsilon_{fu} = 0.01$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-
Bending Moment, $M_a = 7.4461E+007$
Shear Force, $V_a = -25262.081$
EDGE -B-
Bending Moment, $M_b = 1.3414E+006$
Shear Force, $V_b = 25262.081$
BOTH EDGES
Axial Force, $F = -31849.263$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{slt} = 0.00$
-Compression: $A_{slc} = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 2865.133$
-Compression: $A_{sl,com} = 2865.133$
-Middle: $A_{sl,mid} = 615.7522$
Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.33333$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 2.4900E+006$
From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d = 2.4900E+006$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_{w+} + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 816369.853$
 $M_u/V_u - l_w/2 = -1446.901 \leq 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $l_w = 3000.00$
 $M_u = 1.3414E+006$
 $V_u = 25262.081$
 $N_u = 31849.263$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.3195E+006$
 $V_{s1} = 376991.118$ is calculated for pseudo-Column 1, with:
 $d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s2} = 376991.118$ is calculated for pseudo-Column 2, with:
 $d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$
 V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s3} = 565486.678$ is calculated for web, with:
 $d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 500.00$
 V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_f ((11-3)-(11.4), \text{ACI 440}) = 1.5581E+006$
 $f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = \theta_1 + 90^\circ = 90.00$
 $V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, a_1)|)$, with:
 total thickness per orientation, $t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00
 $f_{fe} ((11-5), \text{ACI 440}) = 259.312$
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 250.00$

 End Of Calculation of Shear Capacity for element: wall W1 of floor 1
 At local axis: 3
 Integration Section: (d)

Calculation No. 8

wall W1, Floor 1

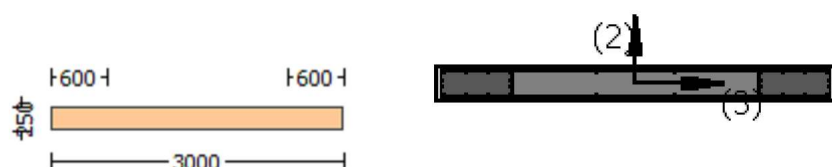
Limit State: Immediate Occupancy (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (μ)

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcrrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou, \min} \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, NoDir = 1
Fiber orientations, bi: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00

Stepwise Properties

At local axis: 3
EDGE -A-
Shear Force, Va = -3.7865323E-029
EDGE -B-
Shear Force, Vb = 3.7865323E-029
BOTH EDGES
Axial Force, F = -27514.027
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: Aslt = 0.00
-Compression: Aslc = 6346.017
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: Asl,ten = 2865.133
-Compression: Asl,com = 2865.133
-Middle: Asl,mid = 0.00
(According to 10.7.2.3 Asl,mid is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 1.77991$
Member Controlled by Shear ($V_e/V_r > 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$
with
 $M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 7.6379E+009$
 $M_{u1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination
 $M_{u1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 7.6379E+009$
 $M_{u2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination
 $M_{u2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of M_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:
 $\phi_u = 1.1557999E-005$
 $M_u = 6.9435E+009$

with full section properties:

b = 250.00
d = 2957.00
d' = 43.00
 $v = 0.00112784$
N = 27514.027
 $f_c = 33.00$
 c_o (5A.5, TBDY) = 0.002
Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: $\phi_u = 0.0035$
 ϕ_{we} ((5.4c), TBDY) = $a_s e^* \phi_{s, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$
where $\phi = a_f * \phi_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
 $\phi_{fx} = 0.00$
 $\phi_{af} = 0.00$

b = 250.00
h = 3000.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.008128$
bw = 250.00
effective stress from (A.35), $ff,e = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $ff,e = 958.8339$

R = 40.00
Effective FRP thickness, $tf = NL*t*\cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase1*A_{col1} + ase2*A_{col2} + ase3*A_{web})/A_{sec} = 0.00$
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00
ase3 = 0 (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
 $As3 = Astir3*ns3 = 0.00$
No stirrups, ns3 = 2.00

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

```

fywe = 694.45
fce = 33.00
From ((5A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 694.45
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfinedsd full section - Steel rupture

```

satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$\mu_u(4.9) = 0.06369787$

$M_u = M_{Rc}(4.14) = 6.9435E+009$

$u = \mu_u(4.1) = 1.1557999E-005$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of μ_{u1} -

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$\mu_u = 1.1645191E-005$

$M_u = 7.6379E+009$

with full section properties:

$b = 250.00$

$d = 2957.00$

$d' = 43.00$

$v = 0.00112784$

$N = 27514.027$

$f_c = 33.00$

$\alpha(5A.5, TBDY) = 0.002$

Final value of μ_u : $\mu_u^* = \text{shear_factor} * \text{Max}(\mu_u, \mu_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\mu_u = 0.0035$

$\mu_{ue}((5.4c), TBDY) = \alpha * \text{sh_min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = \alpha * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web})/A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi_{2,1} = 655400.00$

$\alpha_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 694.45$
 $fce = 33.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 1.00$
 $su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

```

lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
'satisfies Eq. (4.3)
---->
v < vs,y2 - LHS eq.(4.5) is satisfied
---->
su (4.9) = 0.07070831
Mu = MRc (4.14) = 7.6379E+009
u = su (4.1) = 1.1645191E-005

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2+

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 1.1557999E-005
Mu = 6.9435E+009

with full section properties:

b = 250.00
d = 2957.00
d' = 43.00
v = 0.00112784

$N = 27514.027$
 $f_c = 33.00$
 $\alpha (5A.5, TBDY) = 0.002$
 Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\alpha = 0.0035$
 $\alpha_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(\alpha_x, \alpha_y) = 0.00$
 where $\alpha = \alpha_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\alpha_x = 0.00$
 $\alpha_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $f_{fe} = 809.387$

$\alpha_y = 0.00$
 $\alpha_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.00067733$
 $bw = 3000.00$
 effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{f} = 0.015$
 $\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$
 $\alpha_{se1} = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $\alpha_{se2} = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $\alpha_{se3} = 0$ (grid does not provide confinement)
 $p_{sh,\min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$
 Expression ((5.4d), TBDY) for $p_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$
 $p_{s1,x}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.00125664$
 $h_1 = 600.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,x}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.00125664$
 $h_2 = 600.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $p_{s3,x}$ (web) = $(A_{s3} * h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} * n_{s3} = 0.00$
 No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$
 $p_{s1,y}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,y}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.0005236$
 $h_2 = 250.00$

$As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y (web) = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

 $Asec = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 694.45$

$fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c =$ confinement factor $= 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 833.34$

$fy1 = 694.45$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 833.34$

$fy2 = 694.45$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 833.34$

$fyv = 694.45$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 694.45$

with $Esv = Es = 200000.00$

$1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0815606$

$2 = Asl,com / (b * d) * (fs2 / fc) = 0.0815606$

$v = Asl,mid / (b * d) * (fsv / fc) = 0.00$

and confined core properties:

$$b = 190.00$$

$$d = 2927.00$$

$$d' = 13.00$$

$$fcc(5A.2, TBDY) = 33.08129$$

$$cc(5A.5, TBDY) = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$1 = Asl_{ten}/(b*d)*(fs1/fc) = 0.10841651$$

$$2 = Asl_{com}/(b*d)*(fs2/fc) = 0.10841651$$

$$v = Asl_{mid}/(b*d)*(fsv/fc) = 0.00$$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$$su(4.9) = 0.06369787$$

$$\mu_u = MR_c(4.14) = 6.9435E+009$$

$$u = su(4.1) = 1.1557999E-005$$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of μ_{u2} -

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.1645191E-005$$

$$\mu_u = 7.6379E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$fc = 33.00$$

$$co(5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.0035$$

$$v_e((5.4c), TBDY) = a_{se} * sh_{min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u, f = 0.015$$

$$ase((5.4d), TBDY) = (ase1 \cdot A_{col1} + ase2 \cdot A_{col2} + ase3 \cdot A_{web}) / A_{sec} = 0.00$$

$$ase1 = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$ase2 = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$ase3 = 0 \text{ (grid does not provide confinement)}$$

$$psh, min = \min(psh, x, psh, y) = 0.0010472$$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$psh, x = ps1, x + ps2, x + ps3, x = 0.00439823$$

$$ps1, x \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.00125664$$

$$h1 = 600.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2, x \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.00125664$$

$$h2 = 600.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3, x \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00188496$$

$$h3 = 1800.00$$

$$As3 = Astir3 \cdot ns3 = 0.00$$

$$\text{No stirrups, } ns3 = 2.00$$

$$psh, y = ps1, y + ps2, y + ps3, y = 0.0010472$$

$$ps1, y \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.0005236$$

$$h1 = 250.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2, y \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.0005236$$

$$h2 = 250.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3, y \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00$$

$$h3 = 250.00$$

$$As3 = Astir3 \cdot ns3 = 157.0796$$

$$\text{No stirrups, } ns3 = 0.00$$

$$A_{sec} = 750000.00$$

$$s_1 = 100.00$$

$$s_2 = 100.00$$

$$s_3 = 200.00$$

$$f_{ywe} = 694.45$$

$$f_{ce} = 33.00$$

$$\text{From } ((5.A5), TBDY), TBDY: cc = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$y1 = 0.0025$$

$$sh1 = 0.008$$

$$ft1 = 833.34$$

$$fy1 = 694.45$$

$$su1 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$$l_o / l_{ou, min} = l_b / l_d = 1.00$$

$$su1 = 0.4 \cdot esu1_{nominal} ((5.5), TBDY) = 0.032$$

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1, ft1, fy1, it is considered characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 694.45$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/lb, min = 1.00$
 $su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/ld = 1.00$
 $suv = 0.4 \cdot esuv_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.0815606$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.0815606$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.01752837$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc \text{ (5A.2, TBDY)} = 33.08129$
 $cc \text{ (5A.5, TBDY)} = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.10841651$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.10841651$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.02330004$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs, y2$ - LHS eq.(4.5) is satisfied

--->

$su \text{ (4.9)} = 0.07070831$

$Mu = MRc \text{ (4.14)} = 7.6379E+009$

$u = su \text{ (4.1)} = 1.1645191E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Shear Strength $Vr = \text{Min}(Vr1, Vr2) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$\mu_u = 3.1013748E-010$

$V_u = 3.7865323E-029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i ,
as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$b_w = 250.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

Mu = 3.1013748E-010

Vu = 3.7865323E-029

Nu = 27514.027

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$b_w = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 625.00$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$
 Web Width, $W_{web} = 250.00$
 Cover Thickness, $c = 25.00$
 Mean Confinement Factor overall section = 1.00246
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou,min} > 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $ε_{fu} = 0.01$
 Number of directions, $N_{oDir} = 1$
 Fiber orientations, $bi: 0.00^\circ$
 Number of layers, $N_L = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
 EDGE -A-
 Shear Force, $V_a = 1.1832914E-030$
 EDGE -B-
 Shear Force, $V_b = -1.1832914E-030$
 BOTH EDGES
 Axial Force, $F = -27514.027$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $A_{slt} = 0.00$
 -Compression: $A_{slc} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $A_{sl,ten} = 2368.761$
 -Compression: $A_{sl,com} = 2368.761$
 -Middle: $A_{sl,mid} = 0.00$
 (According to 10.7.2.3 $A_{sl,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 0.21417884$
 Member Controlled by Flexure ($V_e/V_r < 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
 with
 $M_{pr1} = \text{Max}(\mu_{u1+}, \mu_{u1-}) = 3.9637E+008$
 $\mu_{u1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination
 $\mu_{u1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(\mu_{u2+}, \mu_{u2-}) = 3.9637E+008$
 $\mu_{u2+} = 3.0844E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the the static loading combination
 $\mu_{u2-} = 3.9637E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:
 $\mu = 0.00018625$

$$\mu = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\alpha (5A.5, \text{TB DY}) = 0.002$$

$$\text{Final value of } \mu: \mu^* = \text{shear_factor} * \text{Max}(\mu, \alpha) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TB DY: } \mu = 0.0035$$

$$w_e ((5.4c), \text{TB DY}) = \alpha * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = \alpha * \mu * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \mu_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$f_y = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \mu_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$\alpha_{se} ((5.4d), \text{TB DY}) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\mu_{sh,\min} = \text{Min}(\mu_{sh,x}, \mu_{sh,y}) = 0.0010472$$

Expression ((5.4d), TB DY) for $\mu_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\mu_{sh,x} = \mu_{s1,x} + \mu_{s2,x} + \mu_{s3,x} = 0.00439823$$

$$\mu_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\mu_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$\mu_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{stir3} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

Asec = 750000.00

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 625.00$

$fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y2, sh2, ft2, fy2$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 \cdot esuv_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered

characteristic value $f_{sv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = f_s = 625.00$
 with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.09490228$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.09490228$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.00$

and confined core properties:

$b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} \text{ (5A.2, TBDY)} = 25.06158$
 $cc \text{ (5A.5, TBDY)} = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture

' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$\mu_u \text{ (4.9)} = 0.17396707$
 $\mu_u = M_{Rc} \text{ (4.14)} = 3.0844E+008$
 $u = \mu_u \text{ (4.1)} = 0.00018625$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of μ_{u1} -

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$u = 0.00019204$
 $\mu_u = 3.9637E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $co \text{ (5A.5, TBDY)} = 0.002$
 Final value of μ_u : $\mu_u^* = \text{shear_factor} \cdot \text{Max}(\mu_u, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\mu_u = 0.0035$
 $\mu_{ue} \text{ ((5.4c), TBDY)} = a_{se} \cdot sh_{min} \cdot f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$
 where $f = a_f \cdot p_f \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $a_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.008128$
 $b_w = 250.00$
 effective stress from (A.35), $f_{fe} = 757.2164$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$

From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $ff,e = 954.4864$

$R = 40.00$
Effective FRP thickness, $tf = NL*t*\cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{,f} = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3*ns3 = 0.00$
No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c =$ confinement factor = 1.00246

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $su_1 = 0.4 * esu_{1,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,
 For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_1 = fs = 625.00$
 with $Es_1 = Es = 200000.00$
 $y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 750.00$
 $fy_2 = 625.00$
 $su_2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_{b,min} = 1.00$
 $su_2 = 0.4 * esu_{2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,
 For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 625.00$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $Min(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b * d) * (fs_1/f_c) = 0.09490228$
 $2 = Asl_{com}/(b * d) * (fs_2/f_c) = 0.09490228$
 $v = Asl_{mid}/(b * d) * (fsv/f_c) = 0.06444293$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b * d) * (fs_1/f_c) = 0.11316025$
 $2 = Asl_{com}/(b * d) * (fs_2/f_c) = 0.11316025$
 $v = Asl_{mid}/(b * d) * (fsv/f_c) = 0.07684091$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)
 --->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.19888934$
 $Mu = MRc (4.14) = 3.9637E+008$
 $u = su (4.1) = 0.00019204$

 Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of μ_{2+}

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 0.00018625$$

$$\mu_u = 3.0844 \times 10^8$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\mu_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \mu_u: \mu_u = \text{shear_factor} * \text{Max}(\mu_o, \mu_o) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu_u = 0.0035$$

$$\mu_{ue} \text{ ((5.4c), TBDY)} = a_{se} * \mu_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\mu_{fx}, \mu_{fy}) = 0.00$$

where $\mu_f = a_f * \mu_{pf} * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\mu_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \mu_{pf} = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\mu_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \mu_{pf} = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$\mu_{u,f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$\mu_{sh,1} = 100.00$$

$$\mu_{bo,1} = 190.00$$

$$\mu_{ho,1} = 540.00$$

$$\mu_{bi2,1} = 655400.00$$

$$a_{se2} = 0.00$$

$$\mu_{sh,2} = 100.00$$

$$\mu_{bo,2} = 190.00$$

$$\mu_{ho,2} = 540.00$$

$$\mu_{bi2,2} = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\mu_{psh,min} = \text{Min}(\mu_{psh,x}, \mu_{psh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\mu_{psh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\mu_{psh,x} = \mu_{ps1,x} + \mu_{ps2,x} + \mu_{ps3,x} = 0.00439823$$

$$\mu_{ps1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,x \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,x \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 * ns3 = 0.00$
 No stirups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 * h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$

```

ftv = 750.00
fyv = 625.00
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lo,min = lb/ld = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 625.00
    with Esv = Es = 200000.00
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
    2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
    c = confinement factor = 1.00246
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
    2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
---->
v < vs,c - RHS eq.(4.5) is satisfied
---->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

```

u = 0.00019204
Mu = 3.9637E+008

```

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
fx = 0.00

```


af = 0.00
b = 250.00
h = 3000.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$
bw = 250.00
effective stress from (A.35), $ff,e = 757.2164$

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $ff,e = 954.4864$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.016$
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015

ase ((5.4d), TBDY) = $(ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$

ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00

ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = $ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
 $As3 = Astir3*ns3 = 0.00$
No stirrups, ns3 = 2.00

psh,y = $ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00

$s_3 = 200.00$
 $f_{ywe} = 625.00$
 $f_{ce} = 25.00$
 From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 750.00$
 $fy_1 = 625.00$
 $su_1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/ld = 1.00$
 $su_1 = 0.4 * esu_1_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu_1_{\text{nominal}} = 0.08$,
 For calculation of esu_1_{nominal} and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_1 = fs = 625.00$
 with $Es_1 = Es = 200000.00$
 $y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 750.00$
 $fy_2 = 625.00$
 $su_2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/lb, \min = 1.00$
 $su_2 = 0.4 * esu_2_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu_2_{\text{nominal}} = 0.08$,
 For calculation of esu_2_{nominal} and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 625.00$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou, \min = lb/ld = 1.00$
 $suv = 0.4 * esuv_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esuv_{\text{nominal}} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{\text{nominal}}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = Asl, \text{ten} / (b * d) * (fs_1 / fc) = 0.09490228$
 $2 = Asl, \text{com} / (b * d) * (fs_2 / fc) = 0.09490228$
 $v = Asl, \text{mid} / (b * d) * (fsv / fc) = 0.06444293$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, \text{TBDY}) = 25.06158$
 $cc (5A.5, \text{TBDY}) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, \text{ten} / (b * d) * (fs_1 / fc) = 0.11316025$
 $2 = Asl, \text{com} / (b * d) * (fs_2 / fc) = 0.11316025$
 $v = Asl, \text{mid} / (b * d) * (fsv / fc) = 0.07684091$

Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

---->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

---->

μ_u (4.9) = 0.19888934

$M_u = M_{Rc}$ (4.14) = 3.9637E+008

$u = \mu_u$ (4.1) = 0.00019204

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / \mu_u - l_w/2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513E-012$

$V_u = 1.1832914E-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s \leq d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s \leq d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b_1 + 90^\circ = 90.00$

$V_f = \min(|V_f(45, \theta)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$
 $M_u/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $M_u = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \alpha_1)|, |V_f(-45, \alpha_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 3
Integration Section: (d)
Section Type: rcrrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$
According to 10.7.2.3, ASCE 41-17, shear walls with
transverse reinforcement percentage, $\gamma < 0.0015$
are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17
 $\gamma = 0.0010472$

with $\gamma = \gamma_1 + \gamma_2 + \gamma_3$, being the shear reinf. ratio in a plane perpendicular to the shear axis 2
(pseudo-col.1 $\gamma_1 = A_{s1} \cdot b_1 / s_1 = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$s_1 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s1} = 157.0796$

(pseudo-col.2 $\gamma_2 = A_{s2} \cdot b_2 / s_2 = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$s_2 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s2} = 157.0796$

(grid $\gamma_3 = A_{s3} \cdot b_3 / s_3 = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$s_3 = 200.00$

total area of hoops perpendicular to shear axis, $A_{s3} = 0.00$

total section area, $A_c = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_b / l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 1.9900347E-010$

Shear Force, $V_2 = -8.1671580E-014$

Shear Force, $V_3 = 25262.081$

Axial Force, $F = -31849.263$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $A_{sc} = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $Asl_{ten} = 2368.761$
-Compression: $Asl_{com} = 2368.761$
-Middle: $Asl_{mid} = 1608.495$

Mean Diameter of Tension Reinforcement, $DbL = 17.20$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.00383542$
 $u = y + p = 0.00383542$

- Calculation of y -

$y = (My \cdot I_p) / (EI)_{Eff} = 0.00183542$ ((10-5), ASCE 41-17))
 $My = 2.8230E+008$
 $(EI)_{Eff} = 0.35 \cdot Ec \cdot I$ (table 10-5)
 $Ec \cdot I = 1.0547E+014$
 $I_p = 0.5 \cdot d = 0.5 \cdot (0.8 \cdot h) = 240.00$

Calculation of Yielding Moment My

Calculation of y and My according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$
 $y_{ten} = 1.5746739E-005$
with $f_y = 500.00$
 $d = 208.00$
 $y = 0.23671621$
 $A = 0.01027198$
 $B = 0.0062138$
with $pt = 0.00379609$
 $pc = 0.00379609$
 $pv = 0.00257772$
 $N = 31849.263$
 $b = 3000.00$
 $" = 0.20192308$
 $y_{comp} = 3.3996131E-005$
with $fc^* (12.3, (ACI 440)) = 25.002$
 $fc = 25.00$
 $fl = 0.17503396$
 $b = 3000.00$
 $h = 250.00$
 $Ag = 750000.00$
From (12.9), ACI 440: $ka = 0.00364754$
 $g = pt + pc + pv = 0.0101699$
 $rc = 40.00$
 $Ae/Ac = 0.52524587$
Effective FRP thickness, $tf = NL \cdot t \cdot \cos(b1) = 1.016$
effective strain from (12.5) and (12.12), $efe = 0.004$
 $fu = 0.01$
 $Ef = 64828.00$
 $Ec = 26999.444$
 $y = 0.23572173$
 $A = 0.01001679$
 $B = 0.00611172$
with $Es = 200000.00$

Calculation of ratio lb/d

Adequate Lap Length: $lb/d \geq 1$

- Calculation of p -

Considering wall controlled by flexure (shear control ratio ≤ 1),

from table 10-19: $p = 0.002$

with:

- Condition i (shear wall and wall segments)

- $(A_s - A_s') \cdot f_y + P / (t_w \cdot l_w \cdot f_c') = -0.1675285$

$A_s = 0.00$

$A_s' = 6346.017$

$f_y = 500.00$

$P = 31849.263$

$t_w = 3000.00$

$l_w = 250.00$

$f_c = 25.00$

- $V / (t_w \cdot l_w \cdot f_c' \cdot 0.5) = 2.6227864E-019$, NOTE: units in lb & in

- Confined Boundary: No

Boundary hoops spacing does not exceed $8d_b$ ($s_1 < 8 \cdot d_b$ and $s_2 < 8 \cdot d_b$)

Boundary Trans. Reinf. does not exceed 50% of ACI 318 provision ($V_{w1} + V_{w2} > 0.50 \cdot (V - V_c - V_{w3})$)

With

Boundary Element 1:

$V_{w1} = 157079.633$

$s_1 = 100.00$

Boundary Element 2:

$V_{w2} = 157079.633$

$s_2 = 100.00$

Grid Shear Force, $V_{w3} = 0.00$

Concrete Shear Force, $V_c = 182995.745$

(The variables above have already been given in Shear control ratio calculation)

Mean diameter of all bars, $d_b = 17.33333$

Design Shear Force, $V = 8.1671580E-014$

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (d)

Calculation No. 9

wall W1, Floor 1

Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{o,min} = l_b/l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-

Bending Moment, $M_a = 3.4241487E-011$

Shear Force, $V_a = 6.5495990E-014$

EDGE -B-

Bending Moment, $M_b = 1.5868888E-010$

Shear Force, $V_b = -6.5495990E-014$

BOTH EDGES

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $Asl_{com} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $Asl_{ten} = 2368.761$
 -Compression: $Asl_{com} = 2368.761$
 -Middle: $Asl_{mid} = 1608.495$
 Mean Diameter of Tension Reinforcement, $DbL_{ten} = 17.20$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 759930.852$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d = 759930.852$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 341651.803$
 $M_u/V_u - l_w/2 = 397.8028 > 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c' \cdot 0.5 \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $M_u = 3.4241487E-011$
 $V_u = 6.5495990E-014$
 $N_u = 30990.641$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$
 $A_v = 0.00$
 $s = 200.00$
 $f_y = 500.00$

V_{s3} has been multiplied by 2($1-s/d$) ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / \text{NoDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 3000.00$

End Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 10

wall W1, Floor 1

Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (μ)

Edge: Start

Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou,min} \geq 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $\epsilon_{fu} = 0.01$
 Number of directions, $NoDir = 1$
 Fiber orientations, $b_i = 0.00^\circ$
 Number of layers, $NL = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3
 EDGE -A-
 Shear Force, $V_a = -3.7865323E-029$
 EDGE -B-
 Shear Force, $V_b = 3.7865323E-029$
 BOTH EDGES
 Axial Force, $F = -27514.027$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $As_t = 0.00$
 -Compression: $As_c = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $As_{t,ten} = 2865.133$
 -Compression: $As_{c,com} = 2865.133$
 -Middle: $As_{mid} = 0.00$
 (According to 10.7.2.3 As_{mid} is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 1.77991$
 Member Controlled by Shear ($V_e/V_r > 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$
 with
 $M_{pr1} = \max(\mu_{u1+}, \mu_{u1-}) = 7.6379E+009$
 $\mu_{u1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
 which is defined for the static loading combination
 $\mu_{u1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
 direction which is defined for the static loading combination
 $M_{pr2} = \max(\mu_{u2+}, \mu_{u2-}) = 7.6379E+009$
 $\mu_{u2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
 which is defined for the the static loading combination
 $\mu_{u2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
 direction which is defined for the the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:
 $\mu_u = 1.1557999E-005$
 $M_u = 6.9435E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$c_o(5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.0035$$

$$w_e((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$a_{se}((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$s_{h,1} = 100.00$$

$$b_{o,1} = 190.00$$

$$h_{o,1} = 540.00$$

$$b_{i2,1} = 655400.00$$

$$a_{se2} = 0.00$$

$$s_{h,2} = 100.00$$

$$b_{o,2} = 190.00$$

$$h_{o,2} = 540.00$$

$$b_{i2,2} = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$p_{sh,min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$$

$$p_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_{,1}) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$p_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_{,2}) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$p_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_{,3}) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{stir3} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$$

$$p_{s1,y} \text{ (column 1)} = (A_{s1} * h_1 / s_{,1}) / A_c = 0.0005236$$

$$h_1 = 250.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$p_{s2,y} \text{ (column 2)} = (A_{s2} * h_2 / s_{,2}) / A_c = 0.0005236$$

$h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y (web) = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

 $Asec = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 694.45$

$fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c =$ confinement factor $= 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 833.34$

$fy1 = 694.45$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/d = 1.00$

$su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1$, $sh1$, $ft1$, $fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1$, $sh1$, $ft1$, $fy1$, are also multiplied by $Min(1, 1.25 * (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 833.34$

$fy2 = 694.45$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2$, $sh2$, $ft2$, $fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1$, $sh1$, $ft1$, $fy1$, are also multiplied by $Min(1, 1.25 * (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 833.34$

$fyv = 694.45$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor $= 1.00$

$lo/lou,min = lb/d = 1.00$

$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv , shv , ftv , fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1$, $sh1$, $ft1$, $fy1$, are also multiplied by $Min(1, 1.25 * (lb/d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 694.45$

with $Esv = Es = 200000.00$

$1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0815606$

$2 = Asl,com / (b * d) * (fs2 / fc) = 0.0815606$

$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.10841651$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.10841651$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $f_c = 33.00$
 $co (5A.5, TBDY) = 0.002$

Final value of $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.0035$

$w_e ((5.4c), TBDY) = a_{se} * sh_{min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$a_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$

$a_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \cos(b1) = 1.016$

$f_u, f = 1055.00$
 $E_f = 64828.00$
 $u, f = 0.015$
 $ase \ ((5.4d), \text{TBDY}) = (ase1 \cdot A_{col1} + ase2 \cdot A_{col2} + ase3 \cdot A_{web}) / A_{sec} = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh, min = \text{Min}(psh, x, psh, y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh, min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh, x = ps1, x + ps2, x + ps3, x = 0.00439823$
 $ps1, x \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2, x \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3, x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh, y = ps1, y + ps2, y + ps3, y = 0.0010472$
 $ps1, y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2, y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3, y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fy_{we} = 694.45$
 $f_{ce} = 33.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00
 $lo/lo_{u, min} = lb/ld = 1.00$
 $su1 = 0.4 \cdot esu1_{nominal} \ ((5.5), \text{TBDY}) = 0.032$
 From table 5A.1, TBDY: $esu1_{nominal} = 0.08$,
 For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered

characteristic value $f_{sy1} = f_{s1}/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{s1} = f_s = 694.45$
 with $E_{s1} = E_s = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{o,min} = l_b/l_{b,min} = 1.00$
 $su2 = 0.4 \cdot esu2_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_{nominal} = 0.08$,
 For calculation of $esu2_{nominal}$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{s2} = f_s = 694.45$
 with $E_{s2} = E_s = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{o,min} = l_b/l_d = 1.00$
 $suv = 0.4 \cdot esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and yv, shv, ftv, fyv , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = f_s = 694.45$
 with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.0815606$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.0815606$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.01752837$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (f_{s1}/f_c) = 0.10841651$
 $2 = A_{sl,com}/(b \cdot d) \cdot (f_{s2}/f_c) = 0.10841651$
 $v = A_{sl,mid}/(b \cdot d) \cdot (f_{sv}/f_c) = 0.02330004$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < v_{s,y2}$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.07070831$

$Mu = MR_c (4.14) = 7.6379E+009$

$u = su (4.1) = 1.1645191E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of Mu_{2+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.1557999E-005$$

$$M_u = 6.9435E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\phi_{co} (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{cc}) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.0035$$

$$\phi_{we} ((5.4c), TBDY) = a_{se} * \phi_{sh,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = a_f * \phi_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f / b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f / b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$a_{se} ((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$a_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{sh,min} = \text{Min}(\phi_{sh,x}, \phi_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{sh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$ps3,x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 694.45$
 $fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.00$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of $Mu2$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $we ((5.4c), TBDY) = ase * sh,min * fywe/fce + Min(fx, fy) = 0.00$
 where $f = af * pf * ffe/fce$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$fx = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $ffe = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.00067733$
 $b_w = 3000.00$
 effective stress from (A.35), $f_{f,e} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$

$a_{se}((5.4d), TBDY) = (a_{se1} \cdot A_{col1} + a_{se2} \cdot A_{col2} + a_{se3} \cdot A_{web}) / A_{sec} = 0.00$
 $a_{se1} = 0.00$

$sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$

$a_{se2} = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$a_{se3} = 0$ (grid does not provide confinement)

$p_{sh,min} = \min(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$
 $p_{s1,x} \text{ (column 1)} = (A_{s1} \cdot h_1 / s_1) / A_c = 0.00125664$
 $h_1 = 600.00$
 $A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,x} \text{ (column 2)} = (A_{s2} \cdot h_2 / s_2) / A_c = 0.00125664$
 $h_2 = 600.00$
 $A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $p_{s3,x} \text{ (web)} = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} \cdot n_{s3} = 0.00$
 No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$
 $p_{s1,y} \text{ (column 1)} = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,y} \text{ (column 2)} = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$
 $h_2 = 250.00$
 $A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $p_{s3,y} \text{ (web)} = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$
 $h_3 = 250.00$
 $A_{s3} = A_{stir3} \cdot n_{s3} = 157.0796$
 No stirrups, $n_{s3} = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$f_{ywe} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), $cc = 0.00202463$

$c =$ confinement factor = 1.00246

$y_1 = 0.0025$

```

sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu1_nominal = 0.08,
    For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
    characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs1 = fs = 694.45
    with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu2_nominal = 0.08,
    For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
    characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs2 = fs = 694.45
    with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 694.45
    with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.07070831

```

$$\begin{aligned} \mu &= M/R_c(4.14) = 7.6379E+009 \\ u &= s_u(4.1) = 1.1645191E-005 \end{aligned}$$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$$\mu_u / V_u - l_w / 2 = 0.00 \leq 0$$

= 1 (normal-weight concrete)

$$f_c' = 33.00, \text{ but } f_c'^{0.5} \leq 8.3 \text{ MPa (22.5.3.1, ACI 318-14)}$$

$$h = 250.00$$

$$d = 2400.00$$

$$l_w = 3000.00$$

$$\mu_u = 3.1013748E-010$$

$$V_u = 3.7865323E-029$$

$$N_u = 27514.027$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$$d = 480.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 555.56$$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$$d = 480.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 555.56$$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$$d = 1440.00$$

$$A_v = 157079.633$$

$$s = 200.00$$

$$f_y = 555.56$$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$$V_f \text{ ((11-3)-(11.4), ACI 440)} = 1.5581E+006$$

$$f = 0.95, \text{ for fully-wrapped sections}$$

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a_i)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

$$\text{orientation 1: } \theta = b_1 + 90^\circ = 90.00$$

$$V_f = \min(|V_f(45, 1)|, |V_f(-45, a_1)|), \text{ with:}$$

$$\text{total thickness per orientation, } t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$$

$$d_{fv} = d \text{ (figure 11.2, ACI 440)} = 2957.00$$

$$f_{fe} \text{ ((11-5), ACI 440)} = 259.312$$

$$E_f = 64828.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.01$$

$$\text{From (11-11), ACI 440: } V_s + V_f \leq 2.2897E+006$$

$$b_w = 250.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$M_u/V_u - l_w/2 = 0.00 \leq 0$

$= 1$ (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$M_u = 3.1013748E+010$

$V_u = 3.7865323E+029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i ,
as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = 45^\circ$ and $\theta = -45^\circ$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, \theta)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$b_w = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\phi = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:
 New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$
 New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 #####
 Note: Especially for the calculation of moment strengths,
 the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
 New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 625.00$
 #####
 Total Height, $H_{tot} = 3000.00$
 Edges Width, $W_{edg} = 250.00$
 Edges Height, $H_{edg} = 600.00$
 Web Width, $W_{web} = 250.00$
 Cover Thickness, $c = 25.00$
 Mean Confinement Factor overall section = 1.00246
 Element Length, $L = 3000.00$
 Secondary Member
 Ribbed Bars
 Ductile Steel
 Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou,min} \geq 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $\epsilon_{fu} = 0.01$
 Number of directions, $N_{oDir} = 1$
 Fiber orientations, $b_i: 0.00^\circ$
 Number of layers, $N_L = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
 EDGE -A-
 Shear Force, $V_a = 1.1832914E-030$
 EDGE -B-
 Shear Force, $V_b = -1.1832914E-030$
 BOTH EDGES
 Axial Force, $F = -27514.027$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $A_{sl,t} = 0.00$
 -Compression: $A_{sl,c} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $A_{sl,ten} = 2368.761$
 -Compression: $A_{sl,com} = 2368.761$
 -Middle: $A_{sl,mid} = 0.00$
 (According to 10.7.2.3 $A_{sl,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 0.21417884$
 Member Controlled by Flexure ($V_e/V_r < 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
 with
 $M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 3.9637E+008$
 $M_{u1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
 which is defined for the static loading combination
 $M_{u1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
 direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 3.9637E+008$

Mu2+ = 3.0844E+008, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

Mu2- = 3.9637E+008, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of Mu1+

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 0.00018625$$

$$M_u = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$c_o \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.0035$$

$$\phi_{we} \text{ ((5.4c), TBDY)} = a_{se} * \phi_{sh, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = a_f * \phi_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\phi_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(\theta_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$a_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{sh, \min} = \text{Min}(\phi_{sh,x}, \phi_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{sh, \min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{sh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$h1 = 600.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,x \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,x \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 * ns3 = 0.00$
 No stirups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 * h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

```

shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lo,min = lb/ld = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 625.00
    with Esv = Es = 200000.00
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
    2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
    c = confinement factor = 1.00246
    1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
    2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
    v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu1-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

```

u = 0.00019204
Mu = 3.9637E+008

```

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
    we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

```

fx = 0.00
af = 0.00
b = 250.00
h = 3000.00
From EC8 A4.4.3(6), pf = 2tf/bw = 0.008128
bw = 250.00
effective stress from (A.35), ff,e = 757.2164

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00
effective stress from (A.35), ff,e = 954.4864

R = 40.00
Effective FRP thickness, tf = NL*t*cos(b1) = 1.016
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Acol1+ase2*Acol2+ase3*Aweb)/Asec = 0.00
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirups, ns2 = 2.00
ps3,y (web) = (As3*h3/s_3)/Ac = 0.00
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00

```

s_2 = 100.00
s_3 = 200.00
fywe = 625.00
fce = 25.00
From ((5A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 750.00
fy1 = 625.00
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/d = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/d)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025

```

$$v = A_{sl, mid} / (b \cdot d) \cdot (f_{sv} / f_c) = 0.07684091$$

Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$$s_u(4.9) = 0.19888934$$

$$\mu_u = M_{Rc}(4.14) = 3.9637E+008$$

$$u = s_u(4.1) = 0.00019204$$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of μ_{u2+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 0.00018625$$

$$\mu_u = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\alpha(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \mu_{cu}: \mu_{cu}^* = \text{shear_factor} \cdot \text{Max}(\mu_{cu}, \mu_{cc}) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu_{cu} = 0.0035$$

$$\mu_{we}((5.4c), TBDY) = \alpha \cdot s_{h, \min} \cdot f_{ywe} / f_{ce} + \text{Min}(\mu_{fx}, \mu_{fy}) = 0.00$$

where $\mu_f = \alpha \cdot p_f \cdot f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\mu_{fx} = 0.00$$

$$\mu_{af} = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$\mu_{fy} = 0.00$$

$$\mu_{af} = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL \cdot t \cdot \cos(\beta_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$\mu_{u,f} = 0.015$$

$$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} \cdot A_{col1} + \alpha_{se2} \cdot A_{col2} + \alpha_{se3} \cdot A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$s_{h_1} = 100.00$$

$$b_{o_1} = 190.00$$

$$h_{o_1} = 540.00$$

$$b_{i2_1} = 655400.00$$

$$\alpha_{se2} = 0.00$$

$sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_{b,min} = 1.00$
 $s_u2 = 0.4 * e_{su2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $e_{su2,nominal} = 0.08$,
 For calculation of $e_{su2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $f_{sy2} = f_{s2}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{s2} = f_s = 625.00$
 with $E_{s2} = E_s = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $s_{uv} = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $s_{uv} = 0.4 * e_{suv,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $e_{suv,nominal} = 0.08$,
 considering characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $e_{suv,nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $f_{syv} = f_{sv}/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $f_{sv} = f_s = 625.00$
 with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b*d) * (f_{s1}/f_c) = 0.09490228$
 $2 = A_{sl,com}/(b*d) * (f_{s2}/f_c) = 0.09490228$
 $v = A_{sl,mid}/(b*d) * (f_{sv}/f_c) = 0.00$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d) * (f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b*d) * (f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b*d) * (f_{sv}/f_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)
 --->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $s_u (4.9) = 0.17396707$
 $M_u = M_{Rc} (4.14) = 3.0844E+008$
 $u = s_u (4.1) = 0.00018625$

 Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

 Calculation of M_{u2} -

 Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 0.00019204$
 $M_u = 3.9637E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$

$d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $\alpha (5A.5, TBDY) = 0.002$
 Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\alpha = 0.0035$
 $\alpha_e ((5.4c), TBDY) = \alpha * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(\alpha_x, \alpha_y) = 0.00$
 where $\alpha = \alpha^* \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\alpha_x = 0.00$
 $\alpha_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $\rho_f = 2t_f / b_w = 0.008128$
 $b_w = 250.00$
 effective stress from (A.35), $f_{fe} = 757.2164$

$\alpha_y = 0.00$
 $\alpha_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $\rho_f = 2t_f / b_w = 0.00067733$
 $b_w = 3000.00$
 effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$
 Effective FRP thickness, $t_f = N L^* t \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{f,f} = 0.015$
 $\alpha_e ((5.4d), TBDY) = (\alpha_e1 * A_{col1} + \alpha_e2 * A_{col2} + \alpha_e3 * A_{web}) / A_{sec} = 0.00$
 $\alpha_e1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $\alpha_e2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $\alpha_e3 = 0$ (grid does not provide confinement)
 $\rho_{sh,\min} = \text{Min}(\rho_{sh,x}, \rho_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $\rho_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$\rho_{sh,x} = \rho_{s1,x} + \rho_{s2,x} + \rho_{s3,x} = 0.00439823$
 $\rho_{s1,x} (\text{column 1}) = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$
 $h_1 = 600.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $\rho_{s2,x} (\text{column 2}) = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$
 $h_2 = 600.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $\rho_{s3,x} (\text{web}) = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} * n_{s3} = 0.00$
 No stirrups, $n_{s3} = 2.00$

$\rho_{sh,y} = \rho_{s1,y} + \rho_{s2,y} + \rho_{s3,y} = 0.0010472$
 $\rho_{s1,y} (\text{column 1}) = (A_{s1} * h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$

$$ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.0005236$$

$$h2 = 250.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirups, } ns2 = 2.00$$

$$ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00$$

$$h3 = 250.00$$

$$As3 = Astir3 \cdot ns3 = 157.0796$$

$$\text{No stirups, } ns3 = 0.00$$

$$A_{sec} = 750000.00$$

$$s_1 = 100.00$$

$$s_2 = 100.00$$

$$s_3 = 200.00$$

$$f_{ywe} = 625.00$$

$$f_{ce} = 25.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$y1 = 0.0025$$

$$sh1 = 0.008$$

$$ft1 = 750.00$$

$$fy1 = 625.00$$

$$su1 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{ou,min} = l_b/l_d = 1.00$$

$$su1 = 0.4 \cdot esu1_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: $esu1_{nominal} = 0.08$,

For calculation of $esu1_{nominal}$ and $y1, sh1, ft1, fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = fs = 625.00$$

$$\text{with } Es1 = Es = 200000.00$$

$$y2 = 0.0025$$

$$sh2 = 0.008$$

$$ft2 = 750.00$$

$$fy2 = 625.00$$

$$su2 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{ou,min} = l_b/l_{b,min} = 1.00$$

$$su2 = 0.4 \cdot esu2_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: $esu2_{nominal} = 0.08$,

For calculation of $esu2_{nominal}$ and $y2, sh2, ft2, fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = fs = 625.00$$

$$\text{with } Es2 = Es = 200000.00$$

$$y_v = 0.0025$$

$$sh_v = 0.008$$

$$ft_v = 750.00$$

$$fy_v = 625.00$$

$$suv = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$l_o/l_{ou,min} = l_b/l_d = 1.00$$

$$suv = 0.4 \cdot esuv_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = fs = 625.00$$

$$\text{with } Es_v = Es = 200000.00$$

$$1 = Asl_{ten} / (b \cdot d) \cdot (fs1 / fc) = 0.09490228$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.09490228$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.06444293$$

and confined core properties:

$$b = 2940.00$$

$$d = 178.00$$

$$d' = 12.00$$

$$f_{cc} (5A.2, TBDY) = 25.06158$$

$$c_c (5A.5, TBDY) = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.07684091$$

Case/Assumption: Unconfined full section - Steel rupture

' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.19888934$$

$$M_u = M_{Rc} (4.14) = 3.9637E+008$$

$$u = s_u (4.1) = 0.00019204$$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83*f'_c*0.5*h*d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f*V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$$M_u/V_u - l_w/2 = 0.00 \leq 0$$

= 1 (normal-weight concrete)

$$f'_c = 25.00, \text{ but } f'_c*0.5 \leq 8.3 \text{ MPa (22.5.3.1, ACI 318-14)}$$

$$h = 3000.00$$

$$d = 200.00$$

$$l_w = 250.00$$

$$M_u = 4.5474513E-012$$

$$V_u = 1.1832914E-030$$

$$N_u = 27514.027$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot\alpha)\sin\alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$

$dfv = d$ (figure 11.2, ACI 440) = 208.00

$ffe((11-5), \text{ACI 440}) = 259.312$

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$bw = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot \alpha^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / \nu_u - l_w / 2 = 0.00 \leq 0$

$= 1$ (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513E-012$

$\nu_u = 1.1832914E-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17, 10.3.4)

$2(1-s/d) = 0.00$

$V_f((11-3)-(11.4), \text{ACI 440}) = 109599.773$

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot\alpha)\sin\alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$

$dfv = d$ (figure 11.2, ACI 440) = 208.00

$ffe((11-5), \text{ACI 440}) = 259.312$

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcrrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

According to 10.7.2.3, ASCE 41-17, shear walls with

transverse reinforcement percentage, $n < 0.0015$

are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17

$n = 0.0010472$

with $n = p_{s1} + p_{s2} + p_{s3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 3

(pseudo-col.1 $p_{s1} = A_{s1} \cdot b_1 / s_1 = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$s_1 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s1} = 157.0796$

(pseudo-col.2 $p_{s2} = A_{s2} \cdot b_2 / s_2 = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$s_2 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s2} = 157.0796$

(grid $p_{s3} = A_{s3} \cdot b_3 / s_3 = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$s_3 = 200.00$

total area of hoops perpendicular to shear axis, $A_{s3} = 0.00$

total section area, $A_c = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_b / d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{ten} = 2865.133$

-Compression: $As_{com} = 2865.133$

-Middle: $As_{mid} = 615.7522$

Mean Diameter of Tension Reinforcement, $Db_L = 17.33333$

Considering wall controlled by Shear (shear control ratio > 1),
interstorey drift provided values are calculated

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.02$
from table 10-20: $u = 0.02$

with:

- Condition i (shear wall and wall segments)

- $(As - As') \cdot f_y + P) / (t_w \cdot l_w \cdot f_c') = -0.16757429$

$As = 0.00$

$As' = 6346.017$

$f_y = 500.00$

$P = 30990.641$

$t_w = 250.00$

$l_w = 3000.00$

$f_c = 25.00$

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Calculation No. 11

wall W1, Floor 1

Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VR_d

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (a)

Section Type: rcwrs

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{o,min} = l_b/l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-

Bending Moment, $M_a = 5.9714E+007$

Shear Force, $V_a = -20258.761$

EDGE -B-

Bending Moment, $M_b = 1.0757E+006$

Shear Force, $V_b = 20258.761$

BOTH EDGES

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $Asl_{com} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $Asl_{ten} = 2865.133$
 -Compression: $Asl_{com} = 2865.133$
 -Middle: $Asl_{mid} = 615.7522$
 Mean Diameter of Tension Reinforcement, $DbL_{ten} = 17.33333$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $VR = 1.0 \cdot V_n = 2.4900E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d = 2.4900E+006$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 782015.611$
 $M_u/V_u - l_w/2 = 1447.552 > 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 25.00$, but $f_c' \cdot 0.5 \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $l_w = 3000.00$
 $M_u = 5.9714E+007$
 $V_u = 20258.761$
 $N_u = 30990.641$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.3195E+006$
 $V_{s1} = 376991.118$ is calculated for pseudo-Column 1, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 376991.118$ is calculated for pseudo-Column 2, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 565486.678$ is calculated for web, with:

$d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 500.00$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections
 $w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = NL \cdot t / \text{NoDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 250.00$

End Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 12

wall W1, Floor 1

Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Chord rotation capacity (ϕ)

Edge: Start

Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 3

(Bending local axis: 2)

Section Type: rcwls

Constant Properties

Knowledge Factor, $\phi = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
 Longitudinal Bars With Ends Lapped Starting at the End Sections
 Adequate Lap Length ($l_o/l_{ou}, \min \geq 1$)
 FRP Wrapping Data
 Type: Carbon
 Cured laminate properties (design values)
 Thickness, $t = 1.016$
 Tensile Strength, $f_{fu} = 1055.00$
 Tensile Modulus, $E_f = 64828.00$
 Elongation, $\epsilon_{fu} = 0.01$
 Number of directions, $N_{oDir} = 1$
 Fiber orientations, $b_i: 0.00^\circ$
 Number of layers, $N_L = 1$
 Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3
 EDGE -A-
 Shear Force, $V_a = -3.7865323E-029$
 EDGE -B-
 Shear Force, $V_b = 3.7865323E-029$
 BOTH EDGES
 Axial Force, $F = -27514.027$
 Longitudinal Reinforcement Area Distribution (in 2 divisions)
 -Tension: $A_{st} = 0.00$
 -Compression: $A_{sc} = 6346.017$
 Longitudinal Reinforcement Area Distribution (in 3 divisions)
 -Tension: $A_{st,ten} = 2865.133$
 -Compression: $A_{st,com} = 2865.133$
 -Middle: $A_{st,mid} = 0.00$
 (According to 10.7.2.3 $A_{st,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 1.77991$
 Member Controlled by Shear ($V_e/V_r > 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$
 with
 $M_{pr1} = \max(\mu_{u1+}, \mu_{u1-}) = 7.6379E+009$
 $\mu_{u1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination
 $\mu_{u1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination
 $M_{pr2} = \max(\mu_{u2+}, \mu_{u2-}) = 7.6379E+009$
 $\mu_{u2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination
 $\mu_{u2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of μ_{u1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:
 $\phi_u = 1.1557999E-005$
 $M_u = 6.9435E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $\nu = 0.00112784$
 $N = 27514.027$

```

fc = 33.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
-----
fx = 0.00
af = 0.00
b = 250.00
h = 3000.00
From EC8 A4.4.3(6), pf = 2tf/bw = 0.008128
bw = 250.00
effective stress from (A.35), ffe = 809.387
-----
fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00
effective stress from (A.35), ffe = 958.8339
-----
R = 40.00
Effective FRP thickness, tf = NL*t*cos(b1) = 1.016
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Acol1+ase2*Acol2+ase3*Aweb)/Asec = 0.00
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00
ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without
earthquake detailing (90° closed stirrups)
-----
psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00
-----
psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796

```

No stirups, ns2 = 2.00
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, ns3 = 0.00

 $A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fy_{we} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou, \min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$

From table 5A.1, TBDY: $esu1_{\text{nominal}} = 0.08$,

For calculation of $esu1_{\text{nominal}}$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou, \min = lb/lb, \min = 1.00$

$su2 = 0.4 \cdot esu2_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$

From table 5A.1, TBDY: $esu2_{\text{nominal}} = 0.08$,

For calculation of $esu2_{\text{nominal}}$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou, \min = lb/ld = 1.00$

$suv = 0.4 \cdot esuv_{\text{nominal}} ((5.5), \text{TBDY}) = 0.032$

From table 5A.1, TBDY: $esuv_{\text{nominal}} = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_{\text{nominal}}$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 694.45$

with $Esv = Es = 200000.00$

$1 = Asl, \text{ten} / (b \cdot d) \cdot (fs1 / fc) = 0.0815606$

$2 = Asl, \text{com} / (b \cdot d) \cdot (fs2 / fc) = 0.0815606$

$v = Asl, \text{mid} / (b \cdot d) \cdot (fsv / fc) = 0.00$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.10841651$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.10841651$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $f_c = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $w_e ((5.4c), TBDY) = a_{se} * sh_{,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$
 where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $a_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $f_{f,e} = 809.387$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$
 $bw = 3000.00$
 effective stress from (A.35), $f_{f,e} = 958.8339$

$R = 40.00$
 Effective FRP thickness, $t_f = NL * t * \text{Cos}(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$

$u, f = 0.015$
 $ase((5.4d), TBDY) = (ase1 \cdot Acol1 + ase2 \cdot Acol2 + ase3 \cdot Aweb) / Asec = 0.00$
 $ase1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh, min = \text{Min}(psh, x, psh, y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh, min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh, x = ps1, x + ps2, x + ps3, x = 0.00439823$
 $ps1, x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2, x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3, x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh, y = ps1, y + ps2, y + ps3, y = 0.0010472$
 $ps1, y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2, y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3, y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 694.45$
 $fce = 33.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lo, min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

```

with fs1 = fs = 694.45
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.07070831
Mu = MRc (4.14) = 7.6379E+009
u = su (4.1) = 1.1645191E-005

```

Calculation of ratio lb/lb

Adequate Lap Length: lb/lb >= 1

Calculation of Mu2+

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.1557999E-005$$

$$Mu = 6.9435E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\alpha (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.0035$$

$$\phi_{we} ((5.4c), TBDY) = \alpha \phi_u * \frac{f_{ywe}}{f_{ce}} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = \alpha \phi_u * \frac{f_{fe}}{f_{ce}}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{sh,min} = \text{Min}(\phi_{sh,x}, \phi_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{sh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$\phi_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

As3 = Astir3*ns3 = 0.00
No stirups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirups, ns2 = 2.00
ps3,y (web) = (As3*h3/s_3)/Ac = 0.00
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

fywe = 694.45
fce = 33.00

From ((5.A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246

y1 = 0.0025
sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = fs = 694.45

with Es1 = Es = 200000.00

y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = fs = 694.45

with Es2 = Es = 200000.00

yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv , shv , ftv , fyv , it is considered
 characteristic value $fsv = fsv/1.2$, from table 5.1, TBDY.
 $y1$, $sh1$, $ft1$, $fy1$, are also multiplied by $Min(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.0815606$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.0815606$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.00$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 fcc (5A.2, TBDY) = 33.08129
 cc (5A.5, TBDY) = 0.00202463
 $c =$ confinement factor = 1.00246
 $1 = Asl,ten / (b * d) * (fs1 / fc) = 0.10841651$
 $2 = Asl,com / (b * d) * (fs2 / fc) = 0.10841651$
 $v = Asl,mid / (b * d) * (fsv / fc) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

---->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied

---->
 su (4.9) = 0.06369787
 $Mu = MRc$ (4.14) = 6.9435E+009
 $u = su$ (4.1) = 1.1557999E-005

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of $Mu2$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$

$fc = 33.00$
 co (5A.5, TBDY) = 0.002

Final value of cu : $cu^* = shear_factor * Max(cu, cc) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $cu = 0.0035$

we ((5.4c), TBDY) = $ase * sh,min * fywe / fce + Min(fx, fy) = 0.00$

where $f = af * pf * ffe / fce$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$fx = 0.00$

$af = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $ffe = 809.387$

$fy = 0.00$

af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $ff,e = 958.8339$

R = 40.00
Effective FRP thickness, $tf = NL*t*\cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u,f = 0.015$
 $ase((5.4d), TBDY) = (ase1*A_{col1} + ase2*A_{col2} + ase3*A_{web})/A_{sec} = 0.00$

ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00

ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
 $As3 = Astir3*ns3 = 0.00$
No stirrups, ns3 = 2.00

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
 $As1 = Astir1*ns1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
 $As2 = Astir2*ns2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
 $As3 = Astir3*ns3 = 157.0796$
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

$f_{ywe} = 694.45$
 $f_{ce} = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
c = confinement factor = 1.00246

y1 = 0.0025
sh1 = 0.008
ft1 = 833.34

```

fy1 = 694.45
su1 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb = 1.00
    su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu1_nominal = 0.08,
    For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
    characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs1 = fs = 694.45
    with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu2_nominal = 0.08,
    For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
    characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs2 = fs = 694.45
    with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 694.45
    with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.07070831
Mu = MRc (4.14) = 7.6379E+009
u = su (4.1) = 1.1645191E-005

```

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 2.8608\text{E}+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$M_u/V_u - l_w/2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$M_u = 3.1013748\text{E}-010$

$V_u = 3.7865323\text{E}-029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661\text{E}+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581\text{E}+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,

where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897\text{E}+006$

$b_w = 250.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c'^{0.5} \cdot h \cdot d$

NOTE: In expression (22.5.1.1) 'Vw' is replaced by 'Vw+ f*Vf' where Vf is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$M_u/V_u - l_w/2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 240.00$

$l_w = 3000.00$

$M_u = 3.1013748E+010$

$V_u = 3.7865323E+029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, a)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $t_{f1} = N_L * t / \text{NoDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 2957.00

f_{fe} ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$b_w = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 625.00$

Total Height, $H_{tot} = 3000.00$
Edges Width, $W_{edg} = 250.00$
Edges Height, $H_{edg} = 600.00$
Web Width, $W_{web} = 250.00$
Cover Thickness, $c = 25.00$
Mean Confinement Factor overall section = 1.00246
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($l_o/l_{ou,min} \geq 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $f_{fu} = 1055.00$
Tensile Modulus, $E_f = 64828.00$
Elongation, $\epsilon_{fu} = 0.01$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2
EDGE -A-
Shear Force, $V_a = 1.1832914E-030$
EDGE -B-
Shear Force, $V_b = -1.1832914E-030$
BOTH EDGES
Axial Force, $F = -27514.027$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{sl,t} = 0.00$
-Compression: $A_{sl,c} = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 2368.761$
-Compression: $A_{sl,com} = 2368.761$
-Middle: $A_{sl,mid} = 0.00$
(According to 10.7.2.3 $A_{sl,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio, $V_e/V_r = 0.21417884$
Member Controlled by Flexure ($V_e/V_r < 1$)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
with
 $M_{pr1} = \max(\mu_{u1+}, \mu_{u1-}) = 3.9637E+008$
 $\mu_{u1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination
 $\mu_{u1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
direction which is defined for the static loading combination
 $M_{pr2} = \max(\mu_{u2+}, \mu_{u2-}) = 3.9637E+008$
 $\mu_{u2+} = 3.0844E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
which is defined for the the static loading combination

Mu2- = 3.9637E+008, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of Mu1+

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 0.00018625$$

$$M_u = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$\phi_{co} \text{ (5A.5, TBDY)} = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu} = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{co}) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.0035$$

$$\phi_{we} \text{ ((5.4c), TBDY)} = \phi_{se} * \phi_{sh,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_{se} = \phi_{af} * \phi_{pf} * \phi_{ffe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$\phi_{af} = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A4.4.3(6), } \phi_{pf} = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } \phi_{ffe} = 757.2164$$

$$\phi_{fy} = 0.00$$

$$\phi_{af} = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A4.4.3(6), } \phi_{pf} = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } \phi_{ffe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$\phi_{ase} \text{ ((5.4d), TBDY)} = (\phi_{ase1} * A_{col1} + \phi_{ase2} * A_{col2} + \phi_{ase3} * A_{web}) / A_{sec} = 0.00$$

$$\phi_{ase1} = 0.00$$

$$\phi_{sh_1} = 100.00$$

$$\phi_{bo_1} = 190.00$$

$$\phi_{ho_1} = 540.00$$

$$\phi_{bi2_1} = 655400.00$$

$$\phi_{ase2} = 0.00$$

$$\phi_{sh_2} = 100.00$$

$$\phi_{bo_2} = 190.00$$

$$\phi_{ho_2} = 540.00$$

$$\phi_{bi2_2} = 655400.00$$

$$\phi_{ase3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{psh,min} = \text{Min}(\phi_{psh,x}, \phi_{psh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{psh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{psh,x} = \phi_{ps1,x} + \phi_{ps2,x} + \phi_{ps3,x} = 0.00439823$$

$$\phi_{ps1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

No stirups, ns1 = 2.00
 $ps2,x \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, ns2 = 2.00
 $ps3,x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirups, ns3 = 2.00

 $psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, ns1 = 2.00
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, ns2 = 2.00
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, ns3 = 0.00

 $Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/l_d = 1.00$
 $su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 625.00$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/l_b,min = 1.00$
 $su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 625.00$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 750.00$

```

fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lo,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu1-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

```

u = 0.00019204
Mu = 3.9637E+008

```

with full section properties:

```

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372
N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
fx = 0.00
af = 0.00

```

b = 250.00
h = 3000.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.008128$
bw = 250.00
effective stress from (A.35), $ff,e = 757.2164$

$f_y = 0.00$
 $a_f = 0.00$
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), $pf = 2tf/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $ff,e = 954.4864$

R = 40.00
Effective FRP thickness, $tf = NL*t*Cos(b1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u,f = 0.015$
 $ase((5.4d), TBDY) = (ase1*Acol1 + ase2*Acol2 + ase3*Aweb)/Asec = 0.00$
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00
ase3 = 0 (grid does not provide confinement)
 $psh,min = Min(psh,x, psh,y) = 0.0010472$
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1*h1/s_1)/Ac = 0.00125664$
h1 = 600.00
 $As1 = Astir1*ns1 = 157.0796$
No stirups, ns1 = 2.00
 $ps2,x$ (column 2) = $(As2*h2/s_2)/Ac = 0.00125664$
h2 = 600.00
 $As2 = Astir2*ns2 = 157.0796$
No stirups, ns2 = 2.00
 $ps3,x$ (web) = $(As3*h3/s_3)/Ac = 0.00188496$
h3 = 1800.00
 $As3 = Astir3*ns3 = 0.00$
No stirups, ns3 = 2.00

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1*h1/s_1)/Ac = 0.0005236$
h1 = 250.00
 $As1 = Astir1*ns1 = 157.0796$
No stirups, ns1 = 2.00
 $ps2,y$ (column 2) = $(As2*h2/s_2)/Ac = 0.0005236$
h2 = 250.00
 $As2 = Astir2*ns2 = 157.0796$
No stirups, ns2 = 2.00
 $ps3,y$ (web) = $(As3*h3/s_3)/Ac = 0.00$
h3 = 250.00
 $As3 = Astir3*ns3 = 157.0796$
No stirups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

```

fywe = 625.00
fce = 25.00
From ((5A.5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 750.00
fy1 = 625.00
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091
Case/Assumption: Unconfinedsd full section - Steel rupture

```

does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$\mu_u(4.9) = 0.19888934$

$M_u = M_{Rc}(4.14) = 3.9637E+008$

$u = \mu_u(4.1) = 0.00019204$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of μ_{u2+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$\mu_u = 0.00018625$

$M_u = 3.0844E+008$

with full section properties:

$b = 3000.00$

$d = 208.00$

$d' = 42.00$

$v = 0.00176372$

$N = 27514.027$

$f_c = 25.00$

$\alpha(5A.5, TBDY) = 0.002$

Final value of μ_u : $\mu_u^* = \text{shear_factor} * \text{Max}(\mu_u, \mu_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\mu_u = 0.0035$

$\mu_{ue}((5.4c), TBDY) = \alpha * \text{sh}_{\min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = \alpha * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 757.2164$

$f_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

$\alpha_{se}((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi_{2,1} = 655400.00$

$\alpha_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/ld = 1.00$
 $su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 625.00$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

```

lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
--->
v < vs,c - RHS eq.(4.5) is satisfied
--->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 0.00019204
Mu = 3.9637E+008

with full section properties:

b = 3000.00
d = 208.00
d' = 42.00
v = 0.00176372

```

N = 27514.027
fc = 25.00
co (5A.5, TBDY) = 0.002
Final value of cu: cu* = shear_factor * Max( cu, cc) = 0.0035
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: cu = 0.0035
we ((5.4c), TBDY) = ase* sh,min*fywe/fce+Min( fx, fy) = 0.00
where f = af*pf*ffe/fce is accounting for FRP contribution like EC8-part3 A.4.4.3(6)
-----
fx = 0.00
af = 0.00
b = 250.00
h = 3000.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.008128
bw = 250.00
effective stress from (A.35), ffe = 757.2164
-----
fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00
effective stress from (A.35), ffe = 954.4864
-----
R = 40.00
Effective FRP thickness, tf = NL*t*cos(b1) = 1.016
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015
ase ((5.4d), TBDY) = (ase1*Acol1+ase2*Acol2+ase3*Aweb)/Asec = 0.00
ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00
ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00
ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472
Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without
earthquake detailing (90° closed stirrups)
-----
psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00
-----
psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00

```


$As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y (web) = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 * esu1_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/lb,min = 1.00$

$su2 = 0.4 * esu2_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY

For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 625.00$

with $Esv = Es = 200000.00$

$1 = Asl,ten / (b * d) * (fs1 / fc) = 0.09490228$

$2 = Asl,com / (b * d) * (fs2 / fc) = 0.09490228$

$v = Asl,mid / (b * d) * (fsv / fc) = 0.06444293$

and confined core properties:

$$b = 2940.00$$

$$d = 178.00$$

$$d' = 12.00$$

$$fcc(5A.2, TBDY) = 25.06158$$

$$cc(5A.5, TBDY) = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.07684091$$

Case/Assumption: Unconfined full section - Steel rupture

' does not satisfy Eq. (4.3)

---->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

---->

$$s_u(4.9) = 0.19888934$$

$$\mu_u = M_{Rc}(4.14) = 3.9637E+008$$

$$u = s_u(4.1) = 0.00019204$$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83*f'_c*0.5*h*d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f*V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$$\mu_u/V_u - l_w/2 = 0.00 \leq 0$$

= 1 (normal-weight concrete)

$$f'_c = 25.00, \text{ but } f_c^{0.5} \leq 8.3 \text{ MPa (22.5.3.1, ACI 318-14)}$$

$$h = 3000.00$$

$$d = 200.00$$

$$l_w = 250.00$$

$$\mu_u = 4.5474513E-012$$

$$V_u = 1.1832914E-030$$

$$N_u = 27514.027$$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f((11-3)-(11.4), ACI 440) = 109599.773$$

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$

$dfv = d$ (figure 11.2, ACI 440) = 208.00

ffe ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$bw = 3000.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$\mu_u / V_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c' \cdot 0.5 \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 4.5474513E-012$

$V_u = 1.1832914E-030$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17, 10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$

$dfv = d$ (figure 11.2, ACI 440) = 208.00

ffe ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$
 $b_w = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
At local axis: 3
Integration Section: (a)
Section Type: rcrrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$
According to 10.7.2.3, ASCE 41-17, shear walls with
transverse reinforcement percentage, $\gamma < 0.0015$
are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17
 $\gamma = 0.0010472$

with $\gamma = \gamma_1 + \gamma_2 + \gamma_3$, being the shear reinf. ratio in a plane perpendicular to the shear axis 2
(pseudo-col.1 $\gamma_1 = A_{s1} \cdot b_1 / s_1 = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $s_1 = 100.00$
total area of hoops perpendicular to shear axis, $A_{s1} = 157.0796$
(pseudo-col.2 $\gamma_2 = A_{s2} \cdot b_2 / s_2 = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$
 $h_2 = 250.00$
 $s_2 = 100.00$
total area of hoops perpendicular to shear axis, $A_{s2} = 157.0796$
(grid $\gamma_3 = A_{s3} \cdot b_3 / s_3 = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$
 $h_3 = 250.00$
 $s_3 = 200.00$
total area of hoops perpendicular to shear axis, $A_{s3} = 0.00$
total section area, $A_c = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$
New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$
Concrete Elasticity, $E_c = 26999.444$
Steel Elasticity, $E_s = 200000.00$
Total Height, $H_{tot} = 3000.00$
Edges Width, $W_{edg} = 250.00$
Edges Height, $H_{edg} = 600.00$
Web Width, $W_{web} = 250.00$
Cover Thickness, $c = 25.00$
Element Length, $L = 3000.00$
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($l_b / d \geq 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $f_{fu} = 1055.00$
Tensile Modulus, $E_f = 64828.00$
Elongation, $\epsilon_{fu} = 0.01$
Number of directions, $N_{oDir} = 1$
Fiber orientations, $b_i: 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 3.4241487\text{E-}011$

Shear Force, $V2 = 6.5495990\text{E-}014$

Shear Force, $V3 = -20258.761$

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{ten} = 2368.761$

-Compression: $As_{com} = 2368.761$

-Middle: $As_{mid} = 1608.495$

Mean Diameter of Tension Reinforcement, $Db_L = 17.20$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.01683488$

$u = y + p = 0.01683488$

- Calculation of y -

$y = (M_y * I_p) / (EI)_{Eff} = 0.00183488$ ((10-5), ASCE 41-17))

$M_y = 2.8221\text{E+}008$

$(EI)_{Eff} = 0.35 * E_c * I$ (table 10-5)

$E_c * I = 1.0547\text{E+}014$

$I_p = 0.5 * d = 0.5 * (0.8 * h) = 240.00$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 1.5745713\text{E-}005$

with $f_y = 500.00$

$d = 208.00$

$y = 0.23666646$

$A = 0.01026923$

$B = 0.00621105$

with $pt = 0.00379609$

$pc = 0.00379609$

$pv = 0.00257772$

$N = 30990.641$

$b = 3000.00$

$" = 0.20192308$

$y_{comp} = 3.3999485\text{E-}005$

with f_c^* (12.3, (ACI 440)) = 25.002

$f_c = 25.00$

$f_l = 0.17503396$

$b = 3000.00$

$h = 250.00$

$Ag = 750000.00$

From (12.9), ACI 440: $ka = 0.00364754$

$g = pt + pc + pv = 0.0101699$

$rc = 40.00$

$Ae/Ac = 0.52524587$

Effective FRP thickness, $t_f = NL * t * \text{Cos}(b1) = 1.016$

effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$

$f_u = 0.01$

$E_f = 64828.00$

$E_c = 26999.444$

$y = 0.23569848$

$A = 0.01002092$

B = 0.00611172
with Es = 200000.00

Calculation of ratio lb/d

Adequate Lap Length: lb/d >= 1

- Calculation of p -

Considering wall controlled by flexure (shear control ratio <= 1),
from table 10-19: p = 0.015

with:

- Condition i (shear wall and wall segments)

- $(A_s - A_s') \cdot f_y + P / (t_w \cdot l_w \cdot f_c') = -0.16757429$

As = 0.00

As' = 6346.017

fy = 500.00

P = 30990.641

tw = 3000.00

lw = 250.00

fc = 25.00

- $V / (t_w \cdot l_w \cdot f_c^{0.5}) = 2.1033264E-019$, NOTE: units in lb & in

- Confined Boundary: No

Boundary hoops spacing does not exceed 8db ($s_1 < 8 \cdot db$ and $s_2 < 8 \cdot db$)

Boundary Trans. Reinf. does not exceed 50% of ACI 318 provision ($V_{w1} + V_{w2} > 0.50 \cdot (V - V_c - V_{w3})$)

With

Boundary Element 1:

Vw1 = 157079.633

s1 = 100.00

Boundary Element 2:

Vw2 = 157079.633

s2 = 100.00

Grid Shear Force, Vw3 = 0.00

Concrete Shear Force, Vc = 341651.803

(The variables above have already been given in Shear control ratio calculation)

Mean diameter of all bars, db = 17.33333

Design Shear Force, V = 6.5495990E-014

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (a)

Calculation No. 13

wall W1, Floor 1

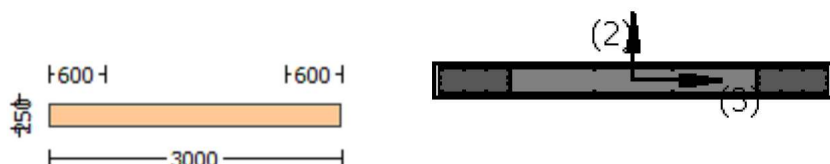
Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity V_{Rd}

Edge: End

Local Axis: (2)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (d)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou,min} = l_b/l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-

Bending Moment, $M_a = 3.4241487\text{E-}011$

Shear Force, $V_a = 6.5495990\text{E-}014$

EDGE -B-

Bending Moment, $M_b = 1.5868888\text{E-}010$

Shear Force, $V_b = -6.5495990\text{E-}014$

BOTH EDGES

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2368.761$

-Compression: $As_{c,com} = 2368.761$

-Middle: $As_{mid} = 1608.495$

Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.20$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 601457.305$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d = 601457.305$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ ' where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 183178.256$

$\mu_u / \mu - l_w / 2 = 2297.879 > 0$

= 1 (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 3000.00$

$d = 200.00$

$l_w = 250.00$

$\mu_u = 1.5868888\text{E-}010$

$V_u = 6.5495990\text{E-}014$

$N_u = 30990.641$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$

$V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:

$d = 200.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 0.00$ is calculated for web, with:

$d = 200.00$

$A_v = 0.00$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$2(1-s/d) = 0.00$

V_f ((11-3)-(11.4), ACI 440) = 109599.773

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot_a) \sin \alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

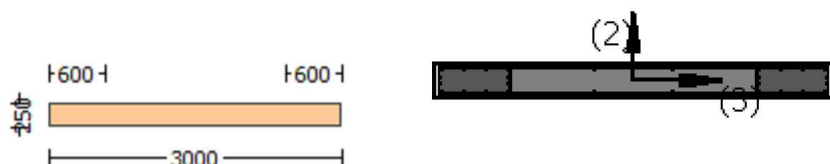
This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading. orientation 1: $\alpha_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:
 total thickness per orientation, $t_{f1} = NL \cdot t / \text{NoDir} = 1.016$
 $d_{fv} = d$ (figure 11.2, ACI 440) = 208.00
 $f_{fe}((11-5), \text{ACI 440}) = 259.312$
 $E_f = 64828.00$
 $f_e = 0.004$, from (11.6a), ACI 440
 with $f_u = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 1.9929\text{E}+006$
 $b_w = 3000.00$

 End Of Calculation of Shear Capacity for element: wall W1 of floor 1
 At local axis: 2
 Integration Section: (d)

Calculation No. 14

wall W1, Floor 1
 Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)
 Analysis: Uniform +X
 Check: Chord rotation capacity (μ)
 Edge: End
 Local Axis: (2)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At Shear local axis: 3
 (Bending local axis: 2)
 Section Type: rcwvs

Constant Properties

 Knowledge Factor, $\gamma = 0.85$
 Mean strength values are used for both shear and moment calculations.
 Consequently:
 New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 694.45$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou,min} > 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 3

EDGE -A-

Shear Force, $V_a = -3.7865323E-029$

EDGE -B-

Shear Force, $V_b = 3.7865323E-029$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten} = 2865.133$

-Compression: $As_{l,com} = 2865.133$

-Middle: $As_{l,mid} = 0.00$

(According to 10.7.2.3 $As_{l,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 1.77991$

Member Controlled by Shear ($V_e/V_r > 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 5.0920E+006$

with

$M_{pr1} = \max(\mu_{u1+}, \mu_{u1-}) = 7.6379E+009$

$\mu_{u1+} = 6.9435E+009$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
which is defined for the static loading combination

$\mu_{u1-} = 7.6379E+009$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
direction which is defined for the static loading combination

$M_{pr2} = \max(\mu_{u2+}, \mu_{u2-}) = 7.6379E+009$

$\mu_{u2+} = 6.9435E+009$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
which is defined for the static loading combination

$\mu_{u2-} = 7.6379E+009$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
direction which is defined for the static loading combination

Calculation of Mu1+

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$\phi_u = 1.1557999E-005$$

$$\mu_u = 6.9435E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\phi_{co} (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_{cu}: \phi_{cu}^* = \text{shear_factor} * \text{Max}(\phi_{cu}, \phi_{co}) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_{cu} = 0.0035$$

$$\phi_{we} ((5.4c), TBDY) = a_{se} * \phi_{sh,min} * f_{ywe} / f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = a_f * \phi_{pf} * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.008128$$

$$b_w = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_{pf} = 2t_f/b_w = 0.00067733$$

$$b_w = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f} = 0.015$$

$$a_{se} ((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$a_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{psh,min} = \text{Min}(\phi_{psh,x}, \phi_{psh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{psh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{psh,x} = \phi_{ps1,x} + \phi_{ps2,x} + \phi_{ps3,x} = 0.00439823$$

$$\phi_{ps1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{ps2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,x} (web) = (As_3 * h_3 / s_3) / Ac = 0.00188496$
 $h_3 = 1800.00$
 $As_3 = Astir_3 * ns_3 = 0.00$
 No stirups, $ns_3 = 2.00$

$psh_y = ps_1_y + ps_2_y + ps_3_y = 0.0010472$
 $ps_{1,y} (column\ 1) = (As_1 * h_1 / s_1) / Ac = 0.0005236$
 $h_1 = 250.00$
 $As_1 = Astir_1 * ns_1 = 157.0796$
 No stirups, $ns_1 = 2.00$
 $ps_{2,y} (column\ 2) = (As_2 * h_2 / s_2) / Ac = 0.0005236$
 $h_2 = 250.00$
 $As_2 = Astir_2 * ns_2 = 157.0796$
 No stirups, $ns_2 = 2.00$
 $ps_{3,y} (web) = (As_3 * h_3 / s_3) / Ac = 0.00$
 $h_3 = 250.00$
 $As_3 = Astir_3 * ns_3 = 157.0796$
 No stirups, $ns_3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fy_{we} = 694.45$
 $f_{ce} = 33.00$

From ((5.A.5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 833.34$
 $fy_1 = 694.45$
 $su_1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou, min = lb/ld = 1.00$

$su_1 = 0.4 * esu_{1,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{1,nominal} = 0.08$,

For calculation of $esu_{1,nominal}$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.

y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_1 = fs = 694.45$

with $Es_1 = Es = 200000.00$

$y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 833.34$
 $fy_2 = 694.45$
 $su_2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$

$lo/lou, min = lb/lb, min = 1.00$

$su_2 = 0.4 * esu_{2,nominal} ((5.5), TBDY) = 0.032$

From table 5A.1, TBDY: $esu_{2,nominal} = 0.08$,

For calculation of $esu_{2,nominal}$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2 / 1.2$, from table 5.1, TBDY.

y_2, sh_2, ft_2, fy_2 , are also multiplied by $\text{Min}(1, 1.25 * (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs_2 = fs = 694.45$

with $Es_2 = Es = 200000.00$

$y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b*d)*(fs1/fc) = 0.0815606$
 $2 = Asl_{com}/(b*d)*(fs2/fc) = 0.0815606$
 $v = Asl_{mid}/(b*d)*(fsv/fc) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b*d)*(fs1/fc) = 0.10841651$
 $2 = Asl_{com}/(b*d)*(fs2/fc) = 0.10841651$
 $v = Asl_{mid}/(b*d)*(fsv/fc) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs_{y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $we ((5.4c), TBDY) = ase * sh_{min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.00$
 where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $fx = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$

bw = 250.00
effective stress from (A.35), $f_{f,e} = 809.387$

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$
bw = 3000.00
effective stress from (A.35), $f_{f,e} = 958.8339$

R = 40.00
Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015
ase ((5.4d), TBDY) = $(ase_1 \cdot A_{col1} + ase_2 \cdot A_{col2} + ase_3 \cdot A_{web}) / A_{sec} = 0.00$

ase1 = 0.00
sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00

ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)

psh,min = Min(psh,x , psh,y) = 0.0010472

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = $ps_{1,x} + ps_{2,x} + ps_{3,x} = 0.00439823$
 $ps_{1,x}$ (column 1) = $(As_1 \cdot h_1 / s_1) / A_c = 0.00125664$
h1 = 600.00
 $As_1 = Astir_1 \cdot ns_1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps_{2,x}$ (column 2) = $(As_2 \cdot h_2 / s_2) / A_c = 0.00125664$
h2 = 600.00
 $As_2 = Astir_2 \cdot ns_2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps_{3,x}$ (web) = $(As_3 \cdot h_3 / s_3) / A_c = 0.00188496$
h3 = 1800.00
 $As_3 = Astir_3 \cdot ns_3 = 0.00$
No stirrups, ns3 = 2.00

psh,y = $ps_{1,y} + ps_{2,y} + ps_{3,y} = 0.0010472$
 $ps_{1,y}$ (column 1) = $(As_1 \cdot h_1 / s_1) / A_c = 0.0005236$
h1 = 250.00
 $As_1 = Astir_1 \cdot ns_1 = 157.0796$
No stirrups, ns1 = 2.00
 $ps_{2,y}$ (column 2) = $(As_2 \cdot h_2 / s_2) / A_c = 0.0005236$
h2 = 250.00
 $As_2 = Astir_2 \cdot ns_2 = 157.0796$
No stirrups, ns2 = 2.00
 $ps_{3,y}$ (web) = $(As_3 \cdot h_3 / s_3) / A_c = 0.00$
h3 = 250.00
 $As_3 = Astir_3 \cdot ns_3 = 157.0796$
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

fywe = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463

```

c = confinement factor = 1.00246
y1 = 0.0025
sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 694.45
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 694.45
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 694.45
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
'satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied

```

--->

$$su(4.9) = 0.07070831$$

$$Mu = MRc(4.14) = 7.6379E+009$$

$$u = su(4.1) = 1.1645191E-005$$

Calculation of ratio lb/d

Adequate Lap Length: $lb/d \geq 1$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.1557999E-005$$

$$Mu = 6.9435E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$co(5A.5, TBDY) = 0.002$$

$$\text{Final value of } cu: cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } cu = 0.0035$$

$$w_e((5.4c), TBDY) = ase * sh_{min} * fy_{we} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = af * pf * ff_e / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$af = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } ff_e = 809.387$$

$$f_y = 0.00$$

$$af = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } pf = 2tf/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } ff_e = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } tf = NL * t * \cos(b1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$ase((5.4d), TBDY) = (ase1 * A_{col1} + ase2 * A_{col2} + ase3 * A_{web}) / A_{sec} = 0.00$$

$$ase1 = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi2_1 = 655400.00$$

$$ase2 = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi2_2 = 655400.00$$

$$ase3 = 0 \text{ (grid does not provide confinement)}$$

$psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 694.45$
 $fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su2 = 0.4 \cdot esu2_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $es_{u2_nominal}$ and y_2 , sh_2, ft_2, fy_2 , it is considered characteristic value $fs_{y2} = fs_2/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 694.45$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 833.34$
 $fy_v = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lo_{u,min} = lb/ld = 1.00$
 $suv = 0.4 \cdot es_{u_nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $es_{u_nominal} = 0.08$,
 considering characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY
 For calculation of $es_{u_nominal}$ and y_v , sh_v, ft_v, fy_v , it is considered characteristic value $fs_{yv} = fs_v/1.2$, from table 5.1, TBDY.
 y_1 , sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_v = fs = 694.45$
 with $Es_v = Es = 200000.00$
 $1 = Asl_{ten}/(b \cdot d) \cdot (fs_1/fc) = 0.0815606$
 $2 = Asl_{com}/(b \cdot d) \cdot (fs_2/fc) = 0.0815606$
 $v = Asl_{mid}/(b \cdot d) \cdot (fs_v/fc) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b \cdot d) \cdot (fs_1/fc) = 0.10841651$
 $2 = Asl_{com}/(b \cdot d) \cdot (fs_2/fc) = 0.10841651$
 $v = Asl_{mid}/(b \cdot d) \cdot (fs_v/fc) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Mu_2 -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$

Final value of c_u : $c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $c_u = 0.0035$

w_e ((5.4c), TBDY) = $a_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = a_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $f_x = 0.00$

$a_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

 $f_y = 0.00$

$a_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

 $R = 40.00$

Effective FRP thickness, $t_f = NL * t * \text{Cos}(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{,f} = 0.015$

a_{se} ((5.4d), TBDY) = $(a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$

$a_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$a_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$a_{se3} = 0$ (grid does not provide confinement)

$p_{sh,\min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

 $p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$

$p_{s1,x}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.00125664$

$h_1 = 600.00$

$A_{s1} = A_{stir1} * n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,x}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.00125664$

$h_2 = 600.00$

$A_{s2} = A_{stir2} * n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,x}$ (web) = $(A_{s3} * h_3 / s_3) / A_c = 0.00188496$

$h_3 = 1800.00$

$A_{s3} = A_{stir3} * n_{s3} = 0.00$

No stirrups, $n_{s3} = 2.00$

 $p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$

$p_{s1,y}$ (column 1) = $(A_{s1} * h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$A_{s1} = A_{stir1} * n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,y}$ (column 2) = $(A_{s2} * h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$A_{s2} = A_{stir2} * n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,y}$ (web) = $(A_{s3} * h_3 / s_3) / A_c = 0.00$

h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirups, ns3 = 0.00

Asec = 750000.00

s_1 = 100.00

s_2 = 100.00

s_3 = 200.00

fywe = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463

c = confinement factor = 1.00246

y1 = 0.0025

sh1 = 0.008

ft1 = 833.34

fy1 = 694.45

su1 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = fs = 694.45

with Es1 = Es = 200000.00

y2 = 0.0025

sh2 = 0.008

ft2 = 833.34

fy2 = 694.45

su2 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = fs = 694.45

with Es2 = Es = 200000.00

yv = 0.0025

shv = 0.008

ftv = 833.34

fyv = 694.45

suv = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb = 1.00

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered

characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = fs = 694.45

with Esv = Es = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606

2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606

v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837

and confined core properties:

b = 190.00

d = 2927.00

$d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.10841651$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.10841651$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.02330004$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)

--->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->

$\mu_u (4.9) = 0.07070831$
 $M_u = M_{Rc} (4.14) = 7.6379E+009$
 $u = \mu_u (4.1) = 1.1645191E-005$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Shear Strength $V_r = \min(V_{r1}, V_{r2}) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83*f_c'^{0.5}*h*d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f*V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$
 $M_u/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $l_w = 3000.00$
 $M_u = 3.1013748E+010$
 $V_u = 3.7865323E+029$
 $N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$
 $V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_f ((11-3)-(11.4), ACI 440) = 1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,

as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, \theta_1)|)$, with:

total thickness per orientation, $tf_1 = NL \cdot t / \text{NoDir} = 1.016$

$df_v = d$ (figure 11.2, ACI 440) = 2957.00

ff_e ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$bw = 250.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '

where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$

$\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$

= 1 (normal-weight concrete)

$f_c' = 33.00$, but $f_c' \cdot 0.5 \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$\mu_u = 3.1013748E-010$

$\mu_u = 3.7865323E-029$

$N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$

$V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin \theta + \cos \theta$ is replaced with $(\cot \theta + \cot \alpha) \sin \alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta, \alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \theta_1)|, |V_f(-45, \theta_1)|)$, with:

total thickness per orientation, $tf_1 = NL \cdot t / \text{NoDir} = 1.016$

$df_v = d$ (figure 11.2, ACI 440) = 2957.00

ff_e ((11-5), ACI 440) = 259.312

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$

$bw = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 3

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At Shear local axis: 2

(Bending local axis: 3)

Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Mean strength values are used for both shear and moment calculations.

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

#####

Note: Especially for the calculation of moment strengths,

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14

New material: Steel Strength, $f_s = 1.25 \cdot f_{sm} = 625.00$

#####

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Mean Confinement Factor overall section = 1.00246

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{ou, min} \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

At local axis: 2

EDGE -A-

Shear Force, $V_a = 1.1832914E-030$

EDGE -B-

Shear Force, $V_b = -1.1832914E-030$

BOTH EDGES

Axial Force, $F = -27514.027$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_{lt} = 0.00$

-Compression: $As_{lc} = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{l,ten} = 2368.761$

-Compression: $As_{l,com} = 2368.761$

-Middle: $As_{l,mid} = 0.00$

(According to 10.7.2.3 $As_{l,mid}$ is setted equal to zero)

Calculation of Shear Capacity ratio , $V_e/V_r = 0.21417884$

Member Controlled by Flexure ($V_e/V_r < 1$)

Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
with

$M_{pr1} = \text{Max}(Mu_{1+}, Mu_{1-}) = 3.9637E+008$

$Mu_{1+} = 3.0844E+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{1-} = 3.9637E+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment direction which is defined for the static loading combination

$M_{pr2} = \text{Max}(Mu_{2+}, Mu_{2-}) = 3.9637E+008$

$Mu_{2+} = 3.0844E+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction which is defined for the static loading combination

$Mu_{2-} = 3.9637E+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment direction which is defined for the static loading combination

Calculation of Mu_{1+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 0.00018625$

$M_u = 3.0844E+008$

with full section properties:

$b = 3000.00$

$d = 208.00$

$d' = 42.00$

$v = 0.00176372$

$N = 27514.027$

$f_c = 25.00$

ϕ_c (5A.5, TBDY) = 0.002

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\phi_u = 0.0035$

ϕ_{we} ((5.4c), TBDY) = $a_{se} * \phi_{sh, \min} * f_{ywe}/f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$

where $\phi_f = a_f * \phi_f^* * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$\phi_{fx} = 0.00$

$a_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $\phi_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 757.2164$

$\phi_{fy} = 0.00$

$a_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $\phi_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = NL * t * \text{Cos}(\theta_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

a_{se} ((5.4d), TBDY) = $(a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web})/A_{sec} = 0.00$

$a_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$
 $ase2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$

```

su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lo,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lo,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
and confined core properties:
b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)
---->
v < vs,c - RHS eq.(4.5) is satisfied
---->
su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu1-

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

u = 0.00019204
Mu = 3.9637E+008

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $\alpha (5A.5, TBDY) = 0.002$
 Final value of α : $\alpha^* = \text{shear_factor} * \text{Max}(\alpha, \alpha_c) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $\alpha = 0.0035$
 $\alpha_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$
 where $f = \alpha * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $\alpha_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f / b_w = 0.008128$
 $b_w = 250.00$
 effective stress from (A.35), $f_{fe} = 757.2164$

$f_y = 0.00$
 $\alpha_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
 From EC8 A.4.4.3(6), $p_f = 2t_f / b_w = 0.00067733$
 $b_w = 3000.00$
 effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$
 Effective FRP thickness, $t_f = N L * t * \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_{,f} = 0.015$
 $\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$
 $\alpha_{se1} = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $\alpha_{se2} = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $\alpha_{se3} = 0$ (grid does not provide confinement)
 $p_{sh,min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$
 $p_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$
 $h_1 = 600.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirrups, $n_{s1} = 2.00$
 $p_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$
 $h_2 = 600.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirrups, $n_{s2} = 2.00$
 $p_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} * n_{s3} = 0.00$
 No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$
 $p_{s1,y} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$

$As1 = Astir1 * ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 * h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 * ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 * h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 * ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

 $Asec = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fywe = 625.00$

$fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

$c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 * esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$

$sh2 = 0.008$

$ft2 = 750.00$

$fy2 = 625.00$

$su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su2 = 0.4 * esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 625.00$

with $Es2 = Es = 200000.00$

$yv = 0.0025$

$shv = 0.008$

$ftv = 750.00$

$fyv = 625.00$

$suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$suv = 0.4 * esuv_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esuv_nominal = 0.08$,

considering characteristic value $fsyv = fsv / 1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fsv = fs = 625.00$

with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.09490228$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.09490228$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.06444293$
and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $f_{cc} (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$
 $2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$
 $v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.07684091$
Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

--->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
--->
 $su (4.9) = 0.19888934$
 $Mu = MR_c (4.14) = 3.9637E+008$
 $u = su (4.1) = 0.00019204$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of Mu_{2+}

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:
 $u = 0.00018625$
 $Mu = 3.0844E+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $co (5A.5, TBDY) = 0.002$
Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
The Shear_factor is considered equal to 1 (pure moment strength)
From (5.4b), TBDY: $cu = 0.0035$
 $w_e ((5.4c), TBDY) = a_{se} * sh_{,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$
where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$
 $a_f = 0.00$
 $b = 250.00$
 $h = 3000.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$
 $bw = 250.00$
effective stress from (A.35), $f_{f,e} = 757.2164$

$f_y = 0.00$
 $a_f = 0.00$
 $b = 3000.00$
 $h = 250.00$
From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00067733$
 $bw = 3000.00$
effective stress from (A.35), $f_{f,e} = 954.4864$

$R = 40.00$
 Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$
 $f_{u,f} = 1055.00$
 $E_f = 64828.00$
 $u_f = 0.015$
 $ase((5.4d), TBDY) = (ase_1 \cdot A_{col1} + ase_2 \cdot A_{col2} + ase_3 \cdot A_{web}) / A_{sec} = 0.00$
 $ase_1 = 0.00$
 $sh_1 = 100.00$
 $bo_1 = 190.00$
 $ho_1 = 540.00$
 $bi2_1 = 655400.00$
 $ase_2 = 0.00$
 $sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$

$ase_3 = 0$ (grid does not provide confinement)
 $psh,min = \min(psh,x, psh,y) = 0.0010472$

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / A_c = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / A_c = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / A_c = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$f_{ywe} = 625.00$
 $f_{ce} = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 c = confinement factor = 1.00246

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lo_{u,min} = lb/ld = 1.00$

```

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu1_nominal = 0.08,
For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs1 = fs = 625.00
with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 750.00
fy2 = 625.00
su2 = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/lb,min = 1.00
su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esu2_nominal = 0.08,
For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fs2 = fs = 625.00
with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 750.00
fyv = 625.00
suv = 0.032
using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00
lo/lou,min = lb/ld = 1.00
suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
From table 5A.1, TBDY: esuv_nominal = 0.08,
considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/ld)^ 2/3), from 10.3.5, ASCE 41-17.
with fsv = fs = 625.00
with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228
2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228
v = Asl,mid/(b*d)*(fsv/fc) = 0.00

```

and confined core properties:

```

b = 2940.00
d = 178.00
d' = 12.00
fcc (5A.2, TBDY) = 25.06158
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025
2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025
v = Asl,mid/(b*d)*(fsv/fc) = 0.00

```

Case/Assumption: Unconfined full section - Steel rupture
' does not satisfy Eq. (4.3)

--->

v < vs,c - RHS eq.(4.5) is satisfied

--->

```

su (4.9) = 0.17396707
Mu = MRc (4.14) = 3.0844E+008
u = su (4.1) = 0.00018625

```

Calculation of ratio lb/ld

Adequate Lap Length: lb/ld >= 1

Calculation of Mu2-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 0.00019204$$
$$Mu = 3.9637E+008$$

with full section properties:

$$b = 3000.00$$
$$d = 208.00$$
$$d' = 42.00$$
$$v = 0.00176372$$
$$N = 27514.027$$
$$f_c = 25.00$$
$$c_o \text{ (5A.5, TBDY)} = 0.002$$
$$\text{Final value of } \mu: \mu^* = \text{shear_factor} * \text{Max}(\mu, c_o) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu = 0.0035$$
$$\mu_e \text{ ((5.4c), TBDY)} = a_{se} * \mu_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\mu_x, \mu_y) = 0.00$$

where $\mu = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\mu_x = 0.00$$
$$a_f = 0.00$$
$$b = 250.00$$
$$h = 3000.00$$

From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.008128$

$$b_w = 250.00$$

effective stress from (A.35), $f_{fe} = 757.2164$

$$\mu_y = 0.00$$
$$a_f = 0.00$$
$$b = 3000.00$$
$$h = 250.00$$

From EC8 A.4.4.3(6), $p_f = 2t_f/b_w = 0.00067733$

$$b_w = 3000.00$$

effective stress from (A.35), $f_{fe} = 954.4864$

$$R = 40.00$$

Effective FRP thickness, $t_f = NL * t * \cos(b_1) = 1.016$

$$f_{u,f} = 1055.00$$
$$E_f = 64828.00$$
$$\mu_{f} = 0.015$$
$$a_{se} \text{ ((5.4d), TBDY)} = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$
$$a_{se1} = 0.00$$
$$\mu_{sh,1} = 100.00$$
$$b_{o,1} = 190.00$$
$$h_{o,1} = 540.00$$
$$b_{i2,1} = 655400.00$$
$$a_{se2} = 0.00$$
$$\mu_{sh,2} = 100.00$$
$$b_{o,2} = 190.00$$
$$h_{o,2} = 540.00$$
$$b_{i2,2} = 655400.00$$
$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$
$$\mu_{sh,min} = \text{Min}(\mu_{sh,x}, \mu_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\mu_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\mu_{sh,x} = \mu_{s1,x} + \mu_{s2,x} + \mu_{s3,x} = 0.00439823$$
$$\mu_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$
$$h_1 = 600.00$$
$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

No stirrups, $n_{s1} = 2.00$

$$\mu_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$h_2 = 600.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirups, $n_{s2} = 2.00$
 $ps_{3,x} (web) = (A_{s3} * h_3 / s_3) / A_c = 0.00188496$
 $h_3 = 1800.00$
 $A_{s3} = A_{stir3} * n_{s3} = 0.00$
 No stirups, $n_{s3} = 2.00$

$p_{sh,y} = ps_1,y + ps_2,y + ps_3,y = 0.0010472$
 $ps_1,y (column\ 1) = (A_{s1} * h_1 / s_1) / A_c = 0.0005236$
 $h_1 = 250.00$
 $A_{s1} = A_{stir1} * n_{s1} = 157.0796$
 No stirups, $n_{s1} = 2.00$
 $ps_2,y (column\ 2) = (A_{s2} * h_2 / s_2) / A_c = 0.0005236$
 $h_2 = 250.00$
 $A_{s2} = A_{stir2} * n_{s2} = 157.0796$
 No stirups, $n_{s2} = 2.00$
 $ps_3,y (web) = (A_{s3} * h_3 / s_3) / A_c = 0.00$
 $h_3 = 250.00$
 $A_{s3} = A_{stir3} * n_{s3} = 157.0796$
 No stirups, $n_{s3} = 0.00$

$A_{sec} = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $f_{ywe} = 625.00$
 $f_{ce} = 25.00$
 From ((5.A.5), TBDY), TBDY: $c_c = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y_1 = 0.0025$
 $sh_1 = 0.008$
 $ft_1 = 750.00$
 $fy_1 = 625.00$
 $su_1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lou,min = lb/l_d = 1.00$
 $su_1 = 0.4 * esu_1_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_1_nominal = 0.08$,
 For calculation of $esu_1_nominal$ and y_1, sh_1, ft_1, fy_1 , it is considered
 characteristic value $fsy_1 = fs_1 / 1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_1 = fs = 625.00$
 with $Es_1 = Es = 200000.00$
 $y_2 = 0.0025$
 $sh_2 = 0.008$
 $ft_2 = 750.00$
 $fy_2 = 625.00$
 $su_2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $Shear_factor = 1.00$
 $lo/lou,min = lb/l_b,min = 1.00$
 $su_2 = 0.4 * esu_2_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu_2_nominal = 0.08$,
 For calculation of $esu_2_nominal$ and y_2, sh_2, ft_2, fy_2 , it is considered
 characteristic value $fsy_2 = fs_2 / 1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs_2 = fs = 625.00$
 with $Es_2 = Es = 200000.00$
 $y_v = 0.0025$
 $sh_v = 0.008$
 $ft_v = 750.00$
 $fy_v = 625.00$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{o,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_{nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_{nominal} = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_{nominal}$ and y_v, sh_v, ft_v, fy_v , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 y_1, sh_1, ft_1, fy_1 , are also multiplied by $\text{Min}(1, 1.25 * (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.09490228$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.09490228$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.06444293$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $fcc (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b*d) * (fs_1/fc) = 0.11316025$
 $2 = Asl_{com}/(b*d) * (fs_2/fc) = 0.11316025$
 $v = Asl_{mid}/(b*d) * (fsv/fc) = 0.07684091$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)
 --->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.19888934$
 $Mu = MRc (4.14) = 3.9637E+008$
 $u = su (4.1) = 0.00019204$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of Shear Strength $V_r = \text{Min}(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83 * fc^{0.5} * h * d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f * V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$
 $Mu/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $fc' = 25.00$, but $fc^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $Mu = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $Nu = 27514.027$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$
 $Av = 157079.633$
 $s = 100.00$
 $fy = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

Vs2 = 157079.633 is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs2 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs3 = 0.00 is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

Vs3 has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f ((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$$f = 0.95, \text{ for fully-wrapped sections}$$

$$w_f/s_f = 1 \text{ (FRP strips adjacent to one another).}$$

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta = b1 + 90^\circ = 90.00$

$$V_f = \text{Min}(|V_f(45, \theta)|, |V_f(-45, a1)|), \text{ with:}$$

$$\text{total thickness per orientation, } t_{f1} = N_L \cdot t / N_{\text{Dir}} = 1.016$$

$$d_{fv} = d \text{ (figure 11.2, ACI 440)} = 208.00$$

$$f_{fe} ((11-5), \text{ACI 440}) = 259.312$$

$$E_f = 64828.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.01$$

$$\text{From (11-11), ACI 440: } V_s + V_f \leq 1.9929E+006$$

$$b_w = 3000.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$

$$\text{From (22.5.1.1) and 11.5.4.3, ACI 318-14: } V_{r2} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '

where V_f is the contribution of FRPs (11.3), ACI 440).

$$\text{From Table (11.5.4.6(d-e)), ACI 318-14: } V_c = 815502.805$$

$$\mu_u / \nu_u - l_w / 2 = 0.00 \leq 0$$

$$= 1 \text{ (normal-weight concrete)}$$

$$f_c' = 25.00, \text{ but } f_c'^{0.5} \leq 8.3 \text{ MPa (22.5.3.1, ACI 318-14)}$$

$$h = 3000.00$$

$$d = 200.00$$

$$l_w = 250.00$$

$$\mu_u = 4.5474513E-012$$

$$\nu_u = 1.1832914E-030$$

$$N_u = 27514.027$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$$

Vs1 = 157079.633 is calculated for pseudo-Column 1, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs1 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs2 = 157079.633 is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs2 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs3 = 0.00 is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

Vs3 has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$f = 0.95$, for fully-wrapped sections

$w_f/s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot\alpha)\sin\alpha$ which is more a generalised expression, where α is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i , as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha_1 = \alpha_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, \alpha_1)|, |V_f(-45, \alpha_1)|)$, with:

total thickness per orientation, $t_{f1} = N_L \cdot t / N_{oDir} = 1.016$

$d_{fv} = d$ (figure 11.2, ACI 440) = 208.00

$f_{fe}((11-5), \text{ACI 440}) = 259.312$

$E_f = 64828.00$

$f_e = 0.004$, from (11.6a), ACI 440

with $f_u = 0.01$

From (11-11), ACI 440: $V_s + V_f \leq 1.9929E+006$

$b_w = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 2

Integration Section: (a)

Section Type: rcrcws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

According to 10.7.2.3, ASCE 41-17, shear walls with

transverse reinforcement percentage, $\rho_n < 0.0015$

are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17

$\rho_n = 0.0010472$

with $\rho_n = \rho_{s1} + \rho_{s2} + \rho_{s3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 3

(pseudo-col.1 $\rho_{s1} = A_{s1} \cdot b_1 / s_1 = (A_{s1} \cdot h_1 / s_1) / A_c = 0.0005236$

$h_1 = 250.00$

$s_1 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s1} = 157.0796$

(pseudo-col.2 $\rho_{s2} = A_{s2} \cdot b_2 / s_2 = (A_{s2} \cdot h_2 / s_2) / A_c = 0.0005236$

$h_2 = 250.00$

$s_2 = 100.00$

total area of hoops perpendicular to shear axis, $A_{s2} = 157.0796$

(grid $\rho_{s3} = A_{s3} \cdot b_3 / s_3 = (A_{s3} \cdot h_3 / s_3) / A_c = 0.00$

$h_3 = 250.00$

$s_3 = 200.00$

total area of hoops perpendicular to shear axis, $A_{s3} = 0.00$

total section area, $A_c = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ($l_b/l_d \geq 1$)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, $t = 1.016$
Tensile Strength, $f_{fu} = 1055.00$
Tensile Modulus, $E_f = 64828.00$
Elongation, $\epsilon_{fu} = 0.01$
Number of directions, $N_{Dir} = 1$
Fiber orientations, $\theta_i = 0.00^\circ$
Number of layers, $N_L = 1$
Radius of rounding corners, $R = 40.00$

Stepwise Properties

Axial Force, $F = -30990.641$
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension: $A_{sl,t} = 0.00$
-Compression: $A_{sl,c} = 6346.017$
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension: $A_{sl,ten} = 2865.133$
-Compression: $A_{sl,com} = 2865.133$
-Middle: $A_{sl,mid} = 615.7522$
Mean Diameter of Tension Reinforcement, $D_{bL} = 17.33333$

Considering wall controlled by Shear (shear control ratio > 1),
interstorey drift provided values are calculated
New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $\phi_{u,R} = 1.0 \times \phi_u = 0.02$
from table 10-20: $\phi_u = 0.02$
with:
- Condition i (shear wall and wall segments)
- $(A_s - A_s') \cdot f_y + P / (t_w \cdot l_w \cdot f_c') = -0.16757429$
 $A_s = 0.00$
 $A_s' = 6346.017$
 $f_y = 500.00$
 $P = 30990.641$
 $t_w = 250.00$
 $l_w = 3000.00$
 $f_c = 25.00$

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1
At local axis: 2
Integration Section: (a)

Calculation No. 15

wall W1, Floor 1

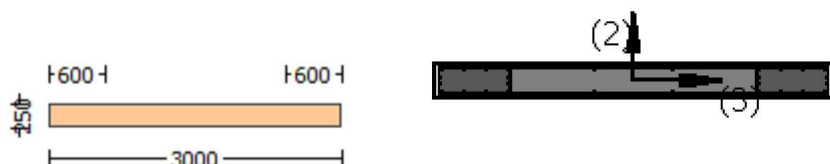
Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)

Analysis: Uniform +X

Check: Shear capacity VR_d

Edge: End

Local Axis: (3)



Start Of Calculation of Shear Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (d)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\gamma = 0.85$

Member Shear Force is generally considered as Force-Controlled Action according to Table C7-1, ASCE 41-17.

Lower-bound strengths are used for Force-Controlled Actions according to 7.5.1.3, ASCE 41-17

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_o/l_{o,min} = l_b/l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i: 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

EDGE -A-

Bending Moment, $M_a = 5.9714\text{E}+007$

Shear Force, $V_a = -20258.761$

EDGE -B-

Bending Moment, $M_b = 1.0757\text{E}+006$

Shear Force, $V_b = 20258.761$

BOTH EDGES

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $As_t = 0.00$

-Compression: $As_c = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $As_{t,ten} = 2865.133$

-Compression: $As_{c,com} = 2865.133$

-Middle: $As_{mid} = 615.7522$

Mean Diameter of Tension Reinforcement, $Db_{L,ten} = 17.33333$

New component: From table 7-7, ASCE 41_17: Final Shear Capacity $V_R = 1.0 \cdot V_n = 2.4900\text{E}+006$

From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_n < 0.83 \cdot f_c' \cdot 0.5 \cdot h \cdot d = 2.4900\text{E}+006$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 816198.128$

$\mu_u / \mu - l_w / 2 = -1446.901 \leq 0$

$= 1$ (normal-weight concrete)

$f_c' = 25.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)

$h = 250.00$

$d = 2400.00$

$l_w = 3000.00$

$\mu_u = 1.0757\text{E}+006$

$V_u = 20258.761$

$N_u = 30990.641$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.3195\text{E}+006$

$V_{s1} = 376991.118$ is calculated for pseudo-Column 1, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s2} = 376991.118$ is calculated for pseudo-Column 2, with:

$d = 480.00$

$A_v = 157079.633$

$s = 100.00$

$f_y = 500.00$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

$V_{s3} = 565486.678$ is calculated for web, with:

$d = 1440.00$

$A_v = 157079.633$

$s = 200.00$

$f_y = 500.00$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581\text{E}+006$

$f = 0.95$, for fully-wrapped sections

$w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(\alpha)$, is implemented for every different fiber orientation α_i ,
as well as for 2 crack directions, $\alpha = 45^\circ$ and $\alpha = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\alpha_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a_1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / NoDir = 1.016$
 $dfv = d$ (figure 11.2, ACI 440) = 2957.00
 ffe ((11-5), ACI 440) = 259.312
 $Ef = 64828.00$
 $fe = 0.004$, from (11.6a), ACI 440
 with $fu = 0.01$
 From (11-11), ACI 440: $Vs + Vf \leq 1.9929E+006$
 $bw = 250.00$

 End Of Calculation of Shear Capacity for element: wall W1 of floor 1
 At local axis: 3
 Integration Section: (d)

Calculation No. 16

wall W1, Floor 1
 Limit State: Collapse Prevention (data interpolation between analysis steps 1 and 2)
 Analysis: Uniform +X
 Check: Chord rotation capacity (μ)
 Edge: End
 Local Axis: (3)



Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At Shear local axis: 3
 (Bending local axis: 2)
 Section Type: rcwrs

Constant Properties

 Knowledge Factor, $\gamma = 0.85$
 Mean strength values are used for both shear and moment calculations.
 Consequently:
 New material of Secondary Member: Concrete Strength, $f_c = f_{cm} = 33.00$
 New material of Secondary Member: Steel Strength, $f_s = f_{sm} = 555.56$
 Concrete Elasticity, $E_c = 26999.444$
 Steel Elasticity, $E_s = 200000.00$
 #####
 Note: Especially for the calculation of moment strengths,


```

the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
New material: Steel Strength, fs = 1.25*fsm = 694.45
#####
Total Height, Htot = 3000.00
Edges Width, Wedg = 250.00
Edges Height, Hedg = 600.00
Web Width, Wweb = 250.00
Cover Thickness, c = 25.00
Mean Confinement Factor overall section = 1.00246
Element Length, L = 3000.00
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length (lo/lou,min>=1)
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness, t = 1.016
Tensile Strength, ffu = 1055.00
Tensile Modulus, Ef = 64828.00
Elongation, efu = 0.01
Number of directions, NoDir = 1
Fiber orientations, bi: 0.00°
Number of layers, NL = 1
Radius of rounding corners, R = 40.00
-----

Stepwise Properties
-----
At local axis: 3
EDGE -A-
Shear Force, Va = -3.7865323E-029
EDGE -B-
Shear Force, Vb = 3.7865323E-029
BOTH EDGES
Axial Force, F = -27514.027
Longitudinal Reinforcement Area Distribution (in 2 divisions)
  -Tension: Aslt = 0.00
  -Compression: Aslc = 6346.017
Longitudinal Reinforcement Area Distribution (in 3 divisions)
  -Tension: Asl,ten = 2865.133
  -Compression: Asl,com = 2865.133
  -Middle: Asl,mid = 0.00
  (According to 10.7.2.3 Asl,mid is setted equal to zero)
-----
-----

Calculation of Shear Capacity ratio , Ve/Vr = 1.77991
Member Controlled by Shear (Ve/Vr > 1)
Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 Ve = (Mpr1 + Mpr2)/ln = 5.0920E+006
with
Mpr1 = Max(Mu1+ , Mu1-) = 7.6379E+009
  Mu1+ = 6.9435E+009, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
  which is defined for the static loading combination
  Mu1- = 7.6379E+009, is the ultimate moment strength at the edge 1 of the member in the opposite moment
  direction which is defined for the static loading combination
Mpr2 = Max(Mu2+ , Mu2-) = 7.6379E+009
  Mu2+ = 6.9435E+009, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
  which is defined for the the static loading combination
  Mu2- = 7.6379E+009, is the ultimate moment strength at the edge 2 of the member in the opposite moment
  direction which is defined for the the static loading combination
-----

Calculation of Mu1+

```

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 1.1557999E-005$$

$$\mu = 6.9435E+009$$

with full section properties:

$$b = 250.00$$

$$d = 2957.00$$

$$d' = 43.00$$

$$v = 0.00112784$$

$$N = 27514.027$$

$$f_c = 33.00$$

$$\alpha_{co} (5A.5, TBDY) = 0.002$$

$$\text{Final value of } \phi_u: \phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \phi_u = 0.0035$$

$$\phi_{we} ((5.4c), TBDY) = \alpha_{se} * \phi_{sh,min} * f_{ywe}/f_{ce} + \text{Min}(\phi_{fx}, \phi_{fy}) = 0.00$$

where $\phi_f = \alpha_f * \phi_f^* f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$\phi_{fx} = 0.00$$

$$\alpha_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 809.387$$

$$\phi_{fy} = 0.00$$

$$\alpha_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } \phi_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 958.8339$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = N L^* t \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} A_{col1} + \alpha_{se2} A_{col2} + \alpha_{se3} A_{web})/A_{sec} = 0.00$$

$$\alpha_{se1} = 0.00$$

$$sh_1 = 100.00$$

$$bo_1 = 190.00$$

$$ho_1 = 540.00$$

$$bi_{2,1} = 655400.00$$

$$\alpha_{se2} = 0.00$$

$$sh_2 = 100.00$$

$$bo_2 = 190.00$$

$$ho_2 = 540.00$$

$$bi_{2,2} = 655400.00$$

$$\alpha_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$\phi_{sh,min} = \text{Min}(\phi_{sh,x}, \phi_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $\phi_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$\phi_{sh,x} = \phi_{s1,x} + \phi_{s2,x} + \phi_{s3,x} = 0.00439823$$

$$\phi_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_1) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$\phi_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_2) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$ps3,x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, $ns1 = 2.00$
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, $ns2 = 2.00$
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$

$fywe = 694.45$
 $fce = 33.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 833.34$
 $fy1 = 694.45$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_d = 1.00$

$su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 694.45$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 833.34$
 $fy2 = 694.45$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/l_b,min = 1.00$

$su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$

From table 5A.1, TBDY: $esu2_nominal = 0.08$,

For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs2 = fs = 694.45$

with $Es2 = Es = 200000.00$

$yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Esv = Es = 200000.00$
 $1 = Asl_{ten}/(b*d)*(fs1/fc) = 0.0815606$
 $2 = Asl_{com}/(b*d)*(fs2/fc) = 0.0815606$
 $v = Asl_{mid}/(b*d)*(fsv/fc) = 0.00$
 and confined core properties:
 $b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $f_{cc} (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl_{ten}/(b*d)*(fs1/fc) = 0.10841651$
 $2 = Asl_{com}/(b*d)*(fs2/fc) = 0.10841651$
 $v = Asl_{mid}/(b*d)*(fsv/fc) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < v_{s,y2}$ - LHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of $Mu1$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} * \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)
 From (5.4b), TBDY: $cu = 0.0035$
 $w_e ((5.4c), TBDY) = ase * sh_{min} * fy_{we}/f_{ce} + \text{Min}(fx, fy) = 0.00$
 where $f = af * pf * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$fx = 0.00$
 $af = 0.00$
 $b = 250.00$
 $h = 3000.00$
 From EC8 A.4.4.3(6), $pf = 2tf/bw = 0.008128$
 $bw = 250.00$
 effective stress from (A.35), $ff_e = 809.387$

fy = 0.00
af = 0.00
b = 3000.00
h = 250.00
From EC8 A.4.4.3(6), pf = 2tf/bw = 0.00067733
bw = 3000.00
effective stress from (A.35), ff,e = 958.8339

R = 40.00
Effective FRP thickness, tf = NL*t*cos(b1) = 1.016
fu,f = 1055.00
Ef = 64828.00
u,f = 0.015

ase ((5.4d), TBDY) = (ase1*Acol1+ase2*Acol2+ase3*Aweb)/Asec = 0.00
ase1 = 0.00

sh_1 = 100.00
bo_1 = 190.00
ho_1 = 540.00
bi2_1 = 655400.00

ase2 = 0.00
sh_2 = 100.00
bo_2 = 190.00
ho_2 = 540.00
bi2_2 = 655400.00

ase3 = 0 (grid does not provide confinement)
psh,min = Min(psh,x , psh,y) = 0.0010472

Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

psh,x = ps1,x+ps2,x+ps3,x = 0.00439823
ps1,x (column 1) = (As1*h1/s_1)/Ac = 0.00125664
h1 = 600.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,x (column 2) = (As2*h2/s_2)/Ac = 0.00125664
h2 = 600.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,x (web) = (As3*h3/s_3)/Ac = 0.00188496
h3 = 1800.00
As3 = Astir3*ns3 = 0.00
No stirrups, ns3 = 2.00

psh,y = ps1,y+ps2,y+ps3,y = 0.0010472
ps1,y (column 1) = (As1*h1/s_1)/Ac = 0.0005236
h1 = 250.00
As1 = Astir1*ns1 = 157.0796
No stirrups, ns1 = 2.00
ps2,y (column 2) = (As2*h2/s_2)/Ac = 0.0005236
h2 = 250.00
As2 = Astir2*ns2 = 157.0796
No stirrups, ns2 = 2.00
ps3,y (web) = (As3*h3/s_3)/Ac = 0.00
h3 = 250.00
As3 = Astir3*ns3 = 157.0796
No stirrups, ns3 = 0.00

Asec = 750000.00
s_1 = 100.00
s_2 = 100.00
s_3 = 200.00

fywe = 694.45
fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463
c = confinement factor = 1.00246

y1 = 0.0025

```

sh1 = 0.008
ft1 = 833.34
fy1 = 694.45
su1 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb = 1.00
    su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu1_nominal = 0.08,
    For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
    characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs1 = fs = 694.45
    with Es1 = Es = 200000.00
y2 = 0.0025
sh2 = 0.008
ft2 = 833.34
fy2 = 694.45
su2 = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb,min = 1.00
    su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esu2_nominal = 0.08,
    For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
    characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fs2 = fs = 694.45
    with Es2 = Es = 200000.00
yv = 0.0025
shv = 0.008
ftv = 833.34
fyv = 694.45
suv = 0.032
    using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
    and also multiplied by the shear_factor according to 15.7.1.4, with
    Shear_factor = 1.00
    lo/lou,min = lb/lb = 1.00
    suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032
    From table 5A.1, TBDY: esuv_nominal = 0.08,
    considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY
    For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
    characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.
    y1, sh1,ft1,fy1, are also multiplied by Min(1,1.25*(lb/lb)^ 2/3), from 10.3.5, ASCE 41-17.
    with fsv = fs = 694.45
    with Esv = Es = 200000.00
1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606
2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606
v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837
and confined core properties:
b = 190.00
d = 2927.00
d' = 13.00
fcc (5A.2, TBDY) = 33.08129
cc (5A.5, TBDY) = 0.00202463
c = confinement factor = 1.00246
1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651
2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651
v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004
Case/Assumption: Unconfined full section - Steel rupture
' satisfies Eq. (4.3)
--->
v < vs,y2 - LHS eq.(4.5) is satisfied
--->
su (4.9) = 0.07070831

```

$\mu_u = M_{Rc} (4.14) = 7.6379E+009$
 $u = s_u (4.1) = 1.1645191E-005$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

Calculation of μ_{u2+}

Calculation of ultimate curvature μ_u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1557999E-005$

$\mu_u = 6.9435E+009$

with full section properties:

$b = 250.00$

$d = 2957.00$

$d' = 43.00$

$v = 0.00112784$

$N = 27514.027$

$f_c = 33.00$

$\alpha_{co} (5A.5, TBDY) = 0.002$

Final value of μ_u : $\mu_u^* = \text{shear_factor} * \text{Max}(\mu_u, \alpha_{co}) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $\mu_u = 0.0035$

$\mu_{we} ((5.4c), TBDY) = \alpha_{se} * \text{sh}_{\min} * f_{ywe} / f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f_x = \alpha_f * p_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$

Effective FRP thickness, $t_f = N L^* t \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

$\alpha_{se} ((5.4d), TBDY) = (\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$\alpha_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$\alpha_{se3} = 0$ (grid does not provide confinement)

$p_{sh,\min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,\min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without

earthquake detailing (90° closed stirrups)

$$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$$

$$ps1,x \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.00125664$$

$$h1 = 600.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2,x \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.00125664$$

$$h2 = 600.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3,x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$$

$$h3 = 1800.00$$

$$As3 = Astir3 \cdot ns3 = 0.00$$

$$\text{No stirrups, } ns3 = 2.00$$

$$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$$

$$ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$$

$$h1 = 250.00$$

$$As1 = Astir1 \cdot ns1 = 157.0796$$

$$\text{No stirrups, } ns1 = 2.00$$

$$ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$$

$$h2 = 250.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirrups, } ns2 = 2.00$$

$$ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$$

$$h3 = 250.00$$

$$As3 = Astir3 \cdot ns3 = 157.0796$$

$$\text{No stirrups, } ns3 = 0.00$$

$$Asec = 750000.00$$

$$s_1 = 100.00$$

$$s_2 = 100.00$$

$$s_3 = 200.00$$

$$fywe = 694.45$$

$$fce = 33.00$$

$$\text{From ((5.A5), TBDY), TBDY: } cc = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$y1 = 0.0025$$

$$sh1 = 0.008$$

$$ft1 = 833.34$$

$$fy1 = 694.45$$

$$su1 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lo_{min} = lb/lb_{min} = 1.00$$

$$su1 = 0.4 \cdot esu1_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu1_{nominal} = 0.08,$$

For calculation of $esu1_{nominal}$ and $y1$, $sh1$, $ft1$, $fy1$, it is considered
characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.

$$y1, sh1, ft1, fy1, \text{ are also multiplied by } \text{Min}(1, 1.25 \cdot (lb/lb_{min})^{2/3}), \text{ from 10.3.5, ASCE 41-17.}$$

$$\text{with } fs1 = fs = 694.45$$

$$\text{with } Es1 = Es = 200000.00$$

$$y2 = 0.0025$$

$$sh2 = 0.008$$

$$ft2 = 833.34$$

$$fy2 = 694.45$$

$$su2 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lo_{min} = lb/lb_{min} = 1.00$$

$$su2 = 0.4 \cdot esu2_{nominal} \text{ ((5.5), TBDY)} = 0.032$$

$$\text{From table 5A.1, TBDY: } esu2_{nominal} = 0.08,$$

For calculation of $esu2_{nominal}$ and $y2$, $sh2$, $ft2$, $fy2$, it is considered
characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 694.45$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 833.34$
 $fyv = 694.45$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/ld = 1.00$
 $suv = 0.4 \cdot esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 694.45$
 with $Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.0815606$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.0815606$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.00$

and confined core properties:

$b = 190.00$
 $d = 2927.00$
 $d' = 13.00$
 $fcc (5A.2, TBDY) = 33.08129$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.10841651$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.10841651$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture

' satisfies Eq. (4.3)

--->

$v < vs, y2$ - LHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.06369787$
 $Mu = MRc (4.14) = 6.9435E+009$
 $u = su (4.1) = 1.1557999E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of $Mu2$ -

Calculation of ultimate curvature u according to 4.1, Biskinis/Fardis 2013:

$u = 1.1645191E-005$
 $Mu = 7.6379E+009$

with full section properties:

$b = 250.00$
 $d = 2957.00$
 $d' = 43.00$
 $v = 0.00112784$
 $N = 27514.027$
 $fc = 33.00$
 $co (5A.5, TBDY) = 0.002$
 Final value of cu : $cu^* = \text{shear_factor} \cdot \text{Max}(cu, cc) = 0.0035$
 The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TBDY: $c_u = 0.0035$

w_e ((5.4c), TBDY) = $a_{se} \cdot sh_{min} \cdot f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$

where $f = a_f \cdot p_f \cdot f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$f_x = 0.00$

$a_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 809.387$

$f_y = 0.00$

$a_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A4.4.3(6), $p_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 958.8339$

$R = 40.00$

Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{,f} = 0.015$

a_{se} ((5.4d), TBDY) = $(a_{se1} \cdot A_{col1} + a_{se2} \cdot A_{col2} + a_{se3} \cdot A_{web})/A_{sec} = 0.00$

$a_{se1} = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$a_{se2} = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$a_{se3} = 0$ (grid does not provide confinement)

$p_{sh,min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$

$p_{s1,x}$ (column 1) = $(A_{s1} \cdot h_1/s_1)/A_c = 0.00125664$

$h_1 = 600.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,x}$ (column 2) = $(A_{s2} \cdot h_2/s_2)/A_c = 0.00125664$

$h_2 = 600.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,x}$ (web) = $(A_{s3} \cdot h_3/s_3)/A_c = 0.00188496$

$h_3 = 1800.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 0.00$

No stirrups, $n_{s3} = 2.00$

$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$

$p_{s1,y}$ (column 1) = $(A_{s1} \cdot h_1/s_1)/A_c = 0.0005236$

$h_1 = 250.00$

$A_{s1} = A_{stir1} \cdot n_{s1} = 157.0796$

No stirrups, $n_{s1} = 2.00$

$p_{s2,y}$ (column 2) = $(A_{s2} \cdot h_2/s_2)/A_c = 0.0005236$

$h_2 = 250.00$

$A_{s2} = A_{stir2} \cdot n_{s2} = 157.0796$

No stirrups, $n_{s2} = 2.00$

$p_{s3,y}$ (web) = $(A_{s3} \cdot h_3/s_3)/A_c = 0.00$

$h_3 = 250.00$

$A_{s3} = A_{stir3} \cdot n_{s3} = 157.0796$

No stirrups, ns3 = 0.00

Asec = 750000.00

s_1 = 100.00

s_2 = 100.00

s_3 = 200.00

fywe = 694.45

fce = 33.00

From ((5.A5), TBDY), TBDY: cc = 0.00202463

c = confinement factor = 1.00246

y1 = 0.0025

sh1 = 0.008

ft1 = 833.34

fy1 = 694.45

su1 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su1 = 0.4*esu1_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs1 = fs = 694.45

with Es1 = Es = 200000.00

y2 = 0.0025

sh2 = 0.008

ft2 = 833.34

fy2 = 694.45

su2 = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

su2 = 0.4*esu2_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fs2 = fs = 694.45

with Es2 = Es = 200000.00

yv = 0.0025

shv = 0.008

ftv = 833.34

fyv = 694.45

suv = 0.032

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

lo/lou,min = lb/lb,min = 1.00

suv = 0.4*esuv_nominal ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/lb)^{2/3})$, from 10.3.5, ASCE 41-17.

with fsv = fs = 694.45

with Esv = Es = 200000.00

1 = Asl,ten/(b*d)*(fs1/fc) = 0.0815606

2 = Asl,com/(b*d)*(fs2/fc) = 0.0815606

v = Asl,mid/(b*d)*(fsv/fc) = 0.01752837

and confined core properties:

b = 190.00

d = 2927.00

d' = 13.00

fcc (5A.2, TBDY) = 33.08129

$cc(5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.10841651$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.10841651$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.02330004$
 Case/Assumption: Unconfined full section - Steel rupture
 ' satisfies Eq. (4.3)
 --->
 $v < vs,y2$ - LHS eq.(4.5) is satisfied
 --->
 $su(4.9) = 0.07070831$
 $Mu = MRc(4.14) = 7.6379E+009$
 $u = su(4.1) = 1.1645191E-005$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Shear Strength $Vr = \text{Min}(Vr1, Vr2) = 2.8608E+006$

Calculation of Shear Strength at edge 1, $Vr1 = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $Vr1 = Vn < 0.83*fc'^{0.5}*h*d$

NOTE: In expression (22.5.1.1) ' Vw ' is replaced by ' $Vw + f*Vf$ '
 where Vf is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $Vc = 936121.954$
 $Mu/Vu-lw/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $fc' = 33.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $lw = 3000.00$
 $Mu = 3.1013748E-010$
 $Vu = 3.7865323E-029$
 $Nu = 27514.027$
 From (11.5.4.8), ACI 318-14: $Vs = Vs1 + Vs2 + Vs3 = 1.4661E+006$
 $Vs1 = 418882.372$ is calculated for pseudo-Column 1, with:
 $d = 480.00$
 $Av = 157079.633$
 $s = 100.00$
 $fy = 555.56$
 $Vs1$ has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $Vs2 = 418882.372$ is calculated for pseudo-Column 2, with:
 $d = 480.00$
 $Av = 157079.633$
 $s = 100.00$
 $fy = 555.56$
 $Vs2$ has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $Vs3 = 628323.557$ is calculated for web, with:
 $d = 1440.00$
 $Av = 157079.633$
 $s = 200.00$
 $fy = 555.56$
 $Vs3$ has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $Vf((11-3)-(11.4), ACI 440) = 1.5581E+006$
 $f = 0.95$, for fully-wrapped sections
 $wf/sf = 1$ (FRP strips adjacent to one another).
 In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a)\sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).
 This later relation, considered as a function $Vf(,)$, is implemented for every different fiber orientation ai ,
 as well as for 2 crack directions, $= 45^\circ$ and $= -45^\circ$ to take into consideration the cyclic seismic loading.
 orientation 1: $\theta_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:
 total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$
 $dfv = d$ (figure 11.2, ACI 440) = 2957.00
 ffe ((11-5), ACI 440) = 259.312
 $E_f = 64828.00$
 $fe = 0.004$, from (11.6a), ACI 440
 with $fu = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$
 $bw = 250.00$

Calculation of Shear Strength at edge 2, $V_{r2} = 2.8608E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r2} = V_n < 0.83 \cdot f_c' \cdot h \cdot d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f \cdot V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 936121.954$
 $\mu_u / \mu_u - l_w / 2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $f_c' = 33.00$, but $f_c'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 250.00$
 $d = 2400.00$
 $l_w = 3000.00$
 $\mu_u = 3.1013748E-010$
 $V_u = 3.7865323E-029$
 $N_u = 27514.027$

From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 1.4661E+006$
 $V_{s1} = 418882.372$ is calculated for pseudo-Column 1, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 555.56$

V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s2} = 418882.372$ is calculated for pseudo-Column 2, with:

$d = 480.00$
 $A_v = 157079.633$
 $s = 100.00$
 $f_y = 555.56$

V_{s2} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

$V_{s3} = 628323.557$ is calculated for web, with:

$d = 1440.00$
 $A_v = 157079.633$
 $s = 200.00$
 $f_y = 555.56$

V_{s3} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17, 10.3.4)

V_f ((11-3)-(11.4), ACI 440) = $1.5581E+006$

$f = 0.95$, for fully-wrapped sections
 $w_f / s_f = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression,
 where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i ,
 as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$V_f = \text{Min}(|V_f(45, 1)|, |V_f(-45, a1)|)$, with:

total thickness per orientation, $tf1 = NL \cdot t / \text{NoDir} = 1.016$
 $dfv = d$ (figure 11.2, ACI 440) = 2957.00
 ffe ((11-5), ACI 440) = 259.312
 $E_f = 64828.00$
 $fe = 0.004$, from (11.6a), ACI 440
 with $fu = 0.01$
 From (11-11), ACI 440: $V_s + V_f \leq 2.2897E+006$
 $bw = 250.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
 At local axis: 3

```

Start Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1
At Shear local axis: 2
(Bending local axis: 3)
Section Type: rcrcws

Constant Properties
-----
Knowledge Factor,  $\phi = 0.85$ 
Mean strength values are used for both shear and moment calculations.
Consequently:
New material of Secondary Member: Concrete Strength,  $f_c = f_{cm} = 25.00$ 
New material of Secondary Member: Steel Strength,  $f_s = f_{sm} = 500.00$ 
Concrete Elasticity,  $E_c = 26999.444$ 
Steel Elasticity,  $E_s = 200000.00$ 
#####
Note: Especially for the calculation of moment strengths,
the above steel re-bar strengths are multiplied by 1.25 according to R18.6.5, ACI 318-14
New material: Steel Strength,  $f_s = 1.25 \cdot f_{sm} = 625.00$ 
#####
Total Height,  $H_{tot} = 3000.00$ 
Edges Width,  $W_{edg} = 250.00$ 
Edges Height,  $H_{edg} = 600.00$ 
Web Width,  $W_{web} = 250.00$ 
Cover Thickness,  $c = 25.00$ 
Mean Confinement Factor overall section = 1.00246
Element Length,  $L = 3000.00$ 
Secondary Member
Ribbed Bars
Ductile Steel
Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)
Longitudinal Bars With Ends Lapped Starting at the End Sections
Adequate Lap Length ( $l_o/l_{ou, min} \geq 1$ )
FRP Wrapping Data
Type: Carbon
Cured laminate properties (design values)
Thickness,  $t = 1.016$ 
Tensile Strength,  $f_{fu} = 1055.00$ 
Tensile Modulus,  $E_f = 64828.00$ 
Elongation,  $\epsilon_{fu} = 0.01$ 
Number of directions,  $N_{oDir} = 1$ 
Fiber orientations,  $b_i: 0.00^\circ$ 
Number of layers,  $N_L = 1$ 
Radius of rounding corners,  $R = 40.00$ 
-----

Stepwise Properties
-----
At local axis: 2
EDGE -A-
Shear Force,  $V_a = 1.1832914E-030$ 
EDGE -B-
Shear Force,  $V_b = -1.1832914E-030$ 
BOTH EDGES
Axial Force,  $F = -27514.027$ 
Longitudinal Reinforcement Area Distribution (in 2 divisions)
-Tension:  $A_{slt} = 0.00$ 
-Compression:  $A_{slc} = 6346.017$ 
Longitudinal Reinforcement Area Distribution (in 3 divisions)
-Tension:  $A_{sl,ten} = 2368.761$ 
-Compression:  $A_{sl,com} = 2368.761$ 
-Middle:  $A_{sl,mid} = 0.00$ 
(According to 10.7.2.3  $A_{sl,mid}$  is setted equal to zero)
-----

```

Calculation of Shear Capacity ratio , $V_e/V_r = 0.21417884$
 Member Controlled by Flexure ($V_e/V_r < 1$)
 Calculation of Shear Demand from fig. R18.6.5, ACI 318-14 $V_e = (M_{pr1} + M_{pr2})/l_n = 264249.963$
 with
 $M_{pr1} = \text{Max}(M_{u1+}, M_{u1-}) = 3.9637\text{E}+008$
 $M_{u1+} = 3.0844\text{E}+008$, is the ultimate moment strength at the edge 1 of the member in the actual moment direction
 which is defined for the static loading combination
 $M_{u1-} = 3.9637\text{E}+008$, is the ultimate moment strength at the edge 1 of the member in the opposite moment
 direction which is defined for the static loading combination
 $M_{pr2} = \text{Max}(M_{u2+}, M_{u2-}) = 3.9637\text{E}+008$
 $M_{u2+} = 3.0844\text{E}+008$, is the ultimate moment strength at the edge 2 of the member in the actual moment direction
 which is defined for the the static loading combination
 $M_{u2-} = 3.9637\text{E}+008$, is the ultimate moment strength at the edge 2 of the member in the opposite moment
 direction which is defined for the the static loading combination

 Calculation of M_{u1+}

 Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:
 $\phi_u = 0.00018625$
 $M_u = 3.0844\text{E}+008$

with full section properties:

$b = 3000.00$
 $d = 208.00$
 $d' = 42.00$
 $v = 0.00176372$
 $N = 27514.027$
 $f_c = 25.00$
 $\alpha_1(5A.5, \text{TB DY}) = 0.002$

Final value of ϕ_u : $\phi_u^* = \text{shear_factor} * \text{Max}(\phi_u, \phi_c) = 0.0035$

The Shear_factor is considered equal to 1 (pure moment strength)

From (5.4b), TB DY: $\phi_u = 0.0035$

ϕ_{we} ((5.4c), TB DY) = $\alpha_1 \epsilon_{sh, \min} * f_{ywe} / f_{ce} + \text{Min}(\phi_x, \phi_y) = 0.00$

where $\phi_f = \alpha_f * \rho_f * f_{fe} / f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

 $\phi_x = 0.00$

$\alpha_f = 0.00$

$b = 250.00$

$h = 3000.00$

From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.008128$

$bw = 250.00$

effective stress from (A.35), $f_{fe} = 757.2164$

$\phi_y = 0.00$

$\alpha_f = 0.00$

$b = 3000.00$

$h = 250.00$

From EC8 A.4.4.3(6), $\rho_f = 2t_f/bw = 0.00067733$

$bw = 3000.00$

effective stress from (A.35), $f_{fe} = 954.4864$

$R = 40.00$

Effective FRP thickness, $t_f = N L * t * \cos(\theta_1) = 1.016$

$f_{u,f} = 1055.00$

$E_f = 64828.00$

$u_{f,f} = 0.015$

α_{se} ((5.4d), TB DY) = $(\alpha_{se1} * A_{col1} + \alpha_{se2} * A_{col2} + \alpha_{se3} * A_{web}) / A_{sec} = 0.00$

$\alpha_{se1} = 0.00$

$sh_{_1} = 100.00$

$bo_{_1} = 190.00$

$ho_{_1} = 540.00$

$bi2_{_1} = 655400.00$

$\alpha_{se2} = 0.00$

$sh_2 = 100.00$
 $bo_2 = 190.00$
 $ho_2 = 540.00$
 $bi2_2 = 655400.00$
 $ase3 = 0$ (grid does not provide confinement)
 $psh,min = \text{Min}(psh,x, psh,y) = 0.0010472$
 Expression ((5.4d), TBDY) for psh,min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh,x = ps1,x + ps2,x + ps3,x = 0.00439823$
 $ps1,x$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.00125664$
 $h1 = 600.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,x$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.00125664$
 $h2 = 600.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,x$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirrups, $ns3 = 2.00$

$psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y$ (column 1) = $(As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirrups, $ns1 = 2.00$
 $ps2,y$ (column 2) = $(As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirrups, $ns2 = 2.00$
 $ps3,y$ (web) = $(As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirrups, $ns3 = 0.00$

$Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$

$y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$

$lo/lou,min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal$ ((5.5), TBDY) = 0.032

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.

$y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

with $fs1 = fs = 625.00$

with $Es1 = Es = 200000.00$

$y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor

and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{o,min} = l_b/l_{b,min} = 1.00$
 $\phi_{s2} = 0.4 \cdot \phi_{s2,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $\phi_{s2,nominal} = 0.08$,
 For calculation of $\phi_{s2,nominal}$ and $\phi_{y2}, \phi_{sh2}, \phi_{ft2}, \phi_{fy2}$, it is considered
 characteristic value $\phi_{sy2} = \phi_{s2}/1.2$, from table 5.1, TBDY.
 $\phi_{y1}, \phi_{sh1}, \phi_{ft1}, \phi_{fy1}$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $\phi_{s2} = \phi_s = 625.00$
 with $E_{s2} = E_s = 200000.00$
 $\phi_{yv} = 0.0025$
 $\phi_{shv} = 0.008$
 $\phi_{ftv} = 750.00$
 $\phi_{fyv} = 625.00$
 $\phi_{suv} = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $l_o/l_{o,min} = l_b/l_d = 1.00$
 $\phi_{suv} = 0.4 \cdot \phi_{suv,nominal} ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $\phi_{suv,nominal} = 0.08$,
 considering characteristic value $\phi_{syv} = \phi_{sv}/1.2$, from table 5.1, TBDY
 For calculation of $\phi_{suv,nominal}$ and $\phi_{yv}, \phi_{shv}, \phi_{ftv}, \phi_{fyv}$, it is considered
 characteristic value $\phi_{syv} = \phi_{sv}/1.2$, from table 5.1, TBDY.
 $\phi_{y1}, \phi_{sh1}, \phi_{ft1}, \phi_{fy1}$, are also multiplied by $\text{Min}(1, 1.25 \cdot (l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $\phi_{sv} = \phi_s = 625.00$
 with $E_{sv} = E_s = 200000.00$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (\phi_{s1}/\phi_c) = 0.09490228$
 $2 = A_{sl,com}/(b \cdot d) \cdot (\phi_{s2}/\phi_c) = 0.09490228$
 $v = A_{sl,mid}/(b \cdot d) \cdot (\phi_{sv}/\phi_c) = 0.00$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $\phi_{cc} (5A.2, TBDY) = 25.06158$
 $\phi_{cc} (5A.5, TBDY) = 0.00202463$
 $\phi_c = \text{confinement factor} = 1.00246$
 $1 = A_{sl,ten}/(b \cdot d) \cdot (\phi_{s1}/\phi_c) = 0.11316025$
 $2 = A_{sl,com}/(b \cdot d) \cdot (\phi_{s2}/\phi_c) = 0.11316025$
 $v = A_{sl,mid}/(b \cdot d) \cdot (\phi_{sv}/\phi_c) = 0.00$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)

---->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 ---->
 $\phi_{su} (4.9) = 0.17396707$
 $\phi_{Mu} = \phi_{MRc} (4.14) = 3.0844E+008$
 $\phi_u = \phi_{su} (4.1) = 0.00018625$

 Calculation of ratio l_b/l_d

 Adequate Lap Length: $l_b/l_d \geq 1$

 Calculation of ϕ_{Mu1} -

 Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$\phi_u = 0.00019204$
 $\phi_{Mu} = 3.9637E+008$

 with full section properties:

$b = 3000.00$
 $d = 208.00$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$c_o(5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, c_c) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.0035$$

$$w_e((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \cos(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{f,f} = 0.015$$

$$a_{se}((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$s_{h,1} = 100.00$$

$$b_{o,1} = 190.00$$

$$h_{o,1} = 540.00$$

$$b_{i2,1} = 655400.00$$

$$a_{se2} = 0.00$$

$$s_{h,2} = 100.00$$

$$b_{o,2} = 190.00$$

$$h_{o,2} = 540.00$$

$$b_{i2,2} = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$p_{sh,min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$$

$$p_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_{h,1}) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$p_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_{h,2}) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

$$\text{No stirrups, } n_{s2} = 2.00$$

$$p_{s3,x} \text{ (web)} = (A_{s3} * h_3 / s_{h,3}) / A_c = 0.00188496$$

$$h_3 = 1800.00$$

$$A_{s3} = A_{stir3} * n_{s3} = 0.00$$

$$\text{No stirrups, } n_{s3} = 2.00$$

$$p_{sh,y} = p_{s1,y} + p_{s2,y} + p_{s3,y} = 0.0010472$$

$$p_{s1,y} \text{ (column 1)} = (A_{s1} * h_1 / s_{h,1}) / A_c = 0.0005236$$

$$h_1 = 250.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$$

$$h2 = 250.00$$

$$As2 = Astir2 \cdot ns2 = 157.0796$$

$$\text{No stirups, } ns2 = 2.00$$

$$ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$$

$$h3 = 250.00$$

$$As3 = Astir3 \cdot ns3 = 157.0796$$

$$\text{No stirups, } ns3 = 0.00$$

$$Asec = 750000.00$$

$$s_1 = 100.00$$

$$s_2 = 100.00$$

$$s_3 = 200.00$$

$$fywe = 625.00$$

$$fce = 25.00$$

$$\text{From } ((5.A5), \text{ TBDY}), \text{ TBDY: } cc = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$y1 = 0.0025$$

$$sh1 = 0.008$$

$$ft1 = 750.00$$

$$fy1 = 625.00$$

$$su1 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/ld = 1.00$$

$$su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: esu1_nominal = 0.08,

For calculation of esu1_nominal and y1, sh1,ft1,fy1, it is considered
characteristic value fsy1 = fs1/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs1 = fs = 625.00$$

$$\text{with } Es1 = Es = 200000.00$$

$$y2 = 0.0025$$

$$sh2 = 0.008$$

$$ft2 = 750.00$$

$$fy2 = 625.00$$

$$su2 = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/lb,min = 1.00$$

$$su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: esu2_nominal = 0.08,

For calculation of esu2_nominal and y2, sh2,ft2,fy2, it is considered
characteristic value fsy2 = fs2/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fs2 = fs = 625.00$$

$$\text{with } Es2 = Es = 200000.00$$

$$yv = 0.0025$$

$$shv = 0.008$$

$$ftv = 750.00$$

$$fyv = 625.00$$

$$suv = 0.032$$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
and also multiplied by the shear_factor according to 15.7.1.4, with
Shear_factor = 1.00

$$lo/lou,min = lb/ld = 1.00$$

$$suv = 0.4 \cdot esuv_nominal \text{ ((5.5), TBDY)} = 0.032$$

From table 5A.1, TBDY: esuv_nominal = 0.08,

considering characteristic value fsyv = fsv/1.2, from table 5.1, TBDY

For calculation of esuv_nominal and yv, shv,ftv,fyv, it is considered
characteristic value fsyv = fsv/1.2, from table 5.1, TBDY.

y1, sh1,ft1,fy1, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.

$$\text{with } fsv = fs = 625.00$$

$$\text{with } Esv = Es = 200000.00$$

$$1 = Asl,ten / (b \cdot d) \cdot (fs1 / fc) = 0.09490228$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.09490228$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.06444293$$

and confined core properties:

$$b = 2940.00$$

$$d = 178.00$$

$$d' = 12.00$$

$$f_{cc} (5A.2, TBDY) = 25.06158$$

$$cc (5A.5, TBDY) = 0.00202463$$

$$c = \text{confinement factor} = 1.00246$$

$$1 = A_{sl,ten}/(b*d)*(f_{s1}/f_c) = 0.11316025$$

$$2 = A_{sl,com}/(b*d)*(f_{s2}/f_c) = 0.11316025$$

$$v = A_{sl,mid}/(b*d)*(f_{sv}/f_c) = 0.07684091$$

Case/Assumption: Unconfined full section - Steel rupture

' does not satisfy Eq. (4.3)

--->

$v < v_{s,c}$ - RHS eq.(4.5) is satisfied

--->

$$s_u (4.9) = 0.19888934$$

$$M_u = M_{Rc} (4.14) = 3.9637E+008$$

$$u = s_u (4.1) = 0.00019204$$

Calculation of ratio l_b/d

Adequate Lap Length: $l_b/d \geq 1$

Calculation of M_{u2+}

Calculation of ultimate curvature ϕ_u according to 4.1, Biskinis/Fardis 2013:

$$u = 0.00018625$$

$$M_u = 3.0844E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$cc (5A.5, TBDY) = 0.002$$

$$\text{Final value of } c_u: c_u^* = \text{shear_factor} * \text{Max}(c_u, cc) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } c_u = 0.0035$$

$$w_e ((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

Effective FRP thickness, $t_f = NL \cdot t \cdot \cos(b_1) = 1.016$

$f_u, f = 1055.00$

$E_f = 64828.00$

$u, f = 0.015$

$ase \ ((5.4d), \text{TBDY}) = (ase1 \cdot A_{col1} + ase2 \cdot A_{col2} + ase3 \cdot A_{web}) / A_{sec} = 0.00$

$ase1 = 0.00$

$sh_1 = 100.00$

$bo_1 = 190.00$

$ho_1 = 540.00$

$bi2_1 = 655400.00$

$ase2 = 0.00$

$sh_2 = 100.00$

$bo_2 = 190.00$

$ho_2 = 540.00$

$bi2_2 = 655400.00$

$ase3 = 0$ (grid does not provide confinement)

$psh, min = \text{Min}(psh, x, psh, y) = 0.0010472$

Expression ((5.4d), TBDY) for psh, min has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$psh, x = ps1, x + ps2, x + ps3, x = 0.00439823$

$ps1, x \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.00125664$

$h1 = 600.00$

$As1 = Astir1 \cdot ns1 = 157.0796$

No stirrups, $ns1 = 2.00$

$ps2, x \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.00125664$

$h2 = 600.00$

$As2 = Astir2 \cdot ns2 = 157.0796$

No stirrups, $ns2 = 2.00$

$ps3, x \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00188496$

$h3 = 1800.00$

$As3 = Astir3 \cdot ns3 = 0.00$

No stirrups, $ns3 = 2.00$

$psh, y = ps1, y + ps2, y + ps3, y = 0.0010472$

$ps1, y \text{ (column 1)} = (As1 \cdot h1 / s_1) / A_c = 0.0005236$

$h1 = 250.00$

$As1 = Astir1 \cdot ns1 = 157.0796$

No stirrups, $ns1 = 2.00$

$ps2, y \text{ (column 2)} = (As2 \cdot h2 / s_2) / A_c = 0.0005236$

$h2 = 250.00$

$As2 = Astir2 \cdot ns2 = 157.0796$

No stirrups, $ns2 = 2.00$

$ps3, y \text{ (web)} = (As3 \cdot h3 / s_3) / A_c = 0.00$

$h3 = 250.00$

$As3 = Astir3 \cdot ns3 = 157.0796$

No stirrups, $ns3 = 0.00$

$A_{sec} = 750000.00$

$s_1 = 100.00$

$s_2 = 100.00$

$s_3 = 200.00$

$fy_{we} = 625.00$

$f_{ce} = 25.00$

From ((5.A5), TBDY), TBDY: $cc = 0.00202463$

c = confinement factor = 1.00246

$y1 = 0.0025$

$sh1 = 0.008$

$ft1 = 750.00$

$fy1 = 625.00$

$su1 = 0.032$

using (30) in Biskinis/Fardis (2013) multiplied with shear_factor and also multiplied by the shear_factor according to 15.7.1.4, with Shear_factor = 1.00

$lo/lou, min = lb/ld = 1.00$

$su1 = 0.4 \cdot esu1_nominal \ ((5.5), \text{TBDY}) = 0.032$

From table 5A.1, TBDY: $esu1_nominal = 0.08$,

For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered characteristic value $fsy1 = fs1/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 625.00$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/lb, min = 1.00$
 $su2 = 0.4 \cdot esu2_nominal \cdot ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered characteristic value $fsy2 = fs2/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 625.00$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 750.00$
 $fyv = 625.00$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 Shear_factor = 1.00
 $lo/lou, min = lb/ld = 1.00$
 $suv = 0.4 \cdot esuv_nominal \cdot ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv, ftv, fyv , it is considered characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/ld)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.09490228$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.09490228$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.00$

and confined core properties:

$b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $fcc (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl, ten / (b \cdot d) \cdot (fs1 / fc) = 0.11316025$
 $2 = Asl, com / (b \cdot d) \cdot (fs2 / fc) = 0.11316025$
 $v = Asl, mid / (b \cdot d) \cdot (fsv / fc) = 0.00$

Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)

--->

$v < vs, c$ - RHS eq.(4.5) is satisfied

--->

$su (4.9) = 0.17396707$
 $Mu = MRc (4.14) = 3.0844E+008$
 $u = su (4.1) = 0.00018625$

Calculation of ratio lb/ld

Adequate Lap Length: $lb/ld \geq 1$

Calculation of Mu2-

Calculation of ultimate curvature μ according to 4.1, Biskinis/Fardis 2013:

$$\mu = 0.00019204$$

$$Mu = 3.9637E+008$$

with full section properties:

$$b = 3000.00$$

$$d = 208.00$$

$$d' = 42.00$$

$$v = 0.00176372$$

$$N = 27514.027$$

$$f_c = 25.00$$

$$cc(5A.5, TBDY) = 0.002$$

$$\text{Final value of } \mu: \mu^* = \text{shear_factor} * \text{Max}(\mu, cc) = 0.0035$$

The Shear_factor is considered equal to 1 (pure moment strength)

$$\text{From (5.4b), TBDY: } \mu = 0.0035$$

$$w_e((5.4c), TBDY) = a_{se} * s_{h,min} * f_{ywe}/f_{ce} + \text{Min}(f_x, f_y) = 0.00$$

where $f = a_f * p_f * f_{fe}/f_{ce}$ is accounting for FRP contribution like EC8-part3 A.4.4.3(6)

$$f_x = 0.00$$

$$a_f = 0.00$$

$$b = 250.00$$

$$h = 3000.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.008128$$

$$bw = 250.00$$

$$\text{effective stress from (A.35), } f_{fe} = 757.2164$$

$$f_y = 0.00$$

$$a_f = 0.00$$

$$b = 3000.00$$

$$h = 250.00$$

$$\text{From EC8 A.4.4.3(6), } p_f = 2t_f/bw = 0.00067733$$

$$bw = 3000.00$$

$$\text{effective stress from (A.35), } f_{fe} = 954.4864$$

$$R = 40.00$$

$$\text{Effective FRP thickness, } t_f = NL * t * \text{Cos}(b_1) = 1.016$$

$$f_{u,f} = 1055.00$$

$$E_f = 64828.00$$

$$u_{,f} = 0.015$$

$$a_{se}((5.4d), TBDY) = (a_{se1} * A_{col1} + a_{se2} * A_{col2} + a_{se3} * A_{web}) / A_{sec} = 0.00$$

$$a_{se1} = 0.00$$

$$s_{h_1} = 100.00$$

$$b_{o_1} = 190.00$$

$$h_{o_1} = 540.00$$

$$b_{i2_1} = 655400.00$$

$$a_{se2} = 0.00$$

$$s_{h_2} = 100.00$$

$$b_{o_2} = 190.00$$

$$h_{o_2} = 540.00$$

$$b_{i2_2} = 655400.00$$

$$a_{se3} = 0 \text{ (grid does not provide confinement)}$$

$$p_{sh,min} = \text{Min}(p_{sh,x}, p_{sh,y}) = 0.0010472$$

Expression ((5.4d), TBDY) for $p_{sh,min}$ has been multiplied by 0.3 according to 15.7.1.3 for members without earthquake detailing (90° closed stirrups)

$$p_{sh,x} = p_{s1,x} + p_{s2,x} + p_{s3,x} = 0.00439823$$

$$p_{s1,x} \text{ (column 1)} = (A_{s1} * h_1 / s_{_1}) / A_c = 0.00125664$$

$$h_1 = 600.00$$

$$A_{s1} = A_{stir1} * n_{s1} = 157.0796$$

$$\text{No stirrups, } n_{s1} = 2.00$$

$$p_{s2,x} \text{ (column 2)} = (A_{s2} * h_2 / s_{_2}) / A_c = 0.00125664$$

$$h_2 = 600.00$$

$$A_{s2} = A_{stir2} * n_{s2} = 157.0796$$

No stirups, ns2 = 2.00
 $ps3,x \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00188496$
 $h3 = 1800.00$
 $As3 = Astir3 \cdot ns3 = 0.00$
 No stirups, ns3 = 2.00

 $psh,y = ps1,y + ps2,y + ps3,y = 0.0010472$
 $ps1,y \text{ (column 1)} = (As1 \cdot h1 / s_1) / Ac = 0.0005236$
 $h1 = 250.00$
 $As1 = Astir1 \cdot ns1 = 157.0796$
 No stirups, ns1 = 2.00
 $ps2,y \text{ (column 2)} = (As2 \cdot h2 / s_2) / Ac = 0.0005236$
 $h2 = 250.00$
 $As2 = Astir2 \cdot ns2 = 157.0796$
 No stirups, ns2 = 2.00
 $ps3,y \text{ (web)} = (As3 \cdot h3 / s_3) / Ac = 0.00$
 $h3 = 250.00$
 $As3 = Astir3 \cdot ns3 = 157.0796$
 No stirups, ns3 = 0.00

 $Asec = 750000.00$
 $s_1 = 100.00$
 $s_2 = 100.00$
 $s_3 = 200.00$
 $fywe = 625.00$
 $fce = 25.00$
 From ((5.A5), TBDY), TBDY: $cc = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $y1 = 0.0025$
 $sh1 = 0.008$
 $ft1 = 750.00$
 $fy1 = 625.00$
 $su1 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/l_d = 1.00$
 $su1 = 0.4 \cdot esu1_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu1_nominal = 0.08$,
 For calculation of $esu1_nominal$ and $y1, sh1, ft1, fy1$, it is considered
 characteristic value $fsy1 = fs1 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs1 = fs = 625.00$
 with $Es1 = Es = 200000.00$
 $y2 = 0.0025$
 $sh2 = 0.008$
 $ft2 = 750.00$
 $fy2 = 625.00$
 $su2 = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with
 $\text{Shear_factor} = 1.00$
 $lo/lou,min = lb/l_b,min = 1.00$
 $su2 = 0.4 \cdot esu2_nominal \text{ ((5.5), TBDY)} = 0.032$
 From table 5A.1, TBDY: $esu2_nominal = 0.08$,
 For calculation of $esu2_nominal$ and $y2, sh2, ft2, fy2$, it is considered
 characteristic value $fsy2 = fs2 / 1.2$, from table 5.1, TBDY.
 $y1, sh1, ft1, fy1$, are also multiplied by $\text{Min}(1, 1.25 \cdot (lb/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fs2 = fs = 625.00$
 with $Es2 = Es = 200000.00$
 $yv = 0.0025$
 $shv = 0.008$
 $ftv = 750.00$
 $fyv = 625.00$
 $suv = 0.032$
 using (30) in Biskinis/Fardis (2013) multiplied with shear_factor
 and also multiplied by the shear_factor according to 15.7.1.4, with

Shear_factor = 1.00
 $l_o/l_{ou,min} = l_b/l_d = 1.00$
 $suv = 0.4 * esuv_nominal ((5.5), TBDY) = 0.032$
 From table 5A.1, TBDY: $esuv_nominal = 0.08$,
 considering characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY
 For calculation of $esuv_nominal$ and yv, shv,ftv,fyv , it is considered
 characteristic value $fsyv = fsv/1.2$, from table 5.1, TBDY.
 $y1, sh1,ft1,fy1$, are also multiplied by $Min(1,1.25*(l_b/l_d)^{2/3})$, from 10.3.5, ASCE 41-17.
 with $fsv = fs = 625.00$
 with $Esv = Es = 200000.00$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.09490228$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.09490228$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.06444293$
 and confined core properties:
 $b = 2940.00$
 $d = 178.00$
 $d' = 12.00$
 $fcc (5A.2, TBDY) = 25.06158$
 $cc (5A.5, TBDY) = 0.00202463$
 $c = \text{confinement factor} = 1.00246$
 $1 = Asl,ten/(b*d)*(fs1/fc) = 0.11316025$
 $2 = Asl,com/(b*d)*(fs2/fc) = 0.11316025$
 $v = Asl,mid/(b*d)*(fsv/fc) = 0.07684091$
 Case/Assumption: Unconfined full section - Steel rupture
 ' does not satisfy Eq. (4.3)
 --->
 $v < v_{s,c}$ - RHS eq.(4.5) is satisfied
 --->
 $su (4.9) = 0.19888934$
 $Mu = MRc (4.14) = 3.9637E+008$
 $u = su (4.1) = 0.00019204$

Calculation of ratio l_b/l_d

Adequate Lap Length: $l_b/l_d \geq 1$

Calculation of Shear Strength $V_r = Min(V_{r1}, V_{r2}) = 1.2338E+006$

Calculation of Shear Strength at edge 1, $V_{r1} = 1.2338E+006$
 From (22.5.1.1) and 11.5.4.3, ACI 318-14: $V_{r1} = V_n < 0.83*fc'^{0.5}*h*d$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f*V_f$ '
 where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$
 $Mu/V_u - l_w/2 = 0.00 \leq 0$
 $= 1$ (normal-weight concrete)
 $fc' = 25.00$, but $fc'^{0.5} \leq 8.3$ MPa (22.5.3.1, ACI 318-14)
 $h = 3000.00$
 $d = 200.00$
 $l_w = 250.00$
 $Mu = 4.5474513E-012$
 $V_u = 1.1832914E-030$
 $Nu = 27514.027$
 From (11.5.4.8), ACI 318-14: $V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$
 $V_{s1} = 157079.633$ is calculated for pseudo-Column 1, with:
 $d = 200.00$
 $Av = 157079.633$
 $s = 100.00$
 $fy = 500.00$
 V_{s1} has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)
 $V_{s2} = 157079.633$ is calculated for pseudo-Column 2, with:
 $d = 200.00$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs2 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs3 = 0.00 is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

Vs3 has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f ((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$$f = 0.95, \text{ for fully-wrapped sections}$$

$$w_f/s_f = 1 \text{ (FRP strips adjacent to one another).}$$

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $V_f(,)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $a = 45^\circ$ and $a = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $a_1 = b_1 + 90^\circ = 90.00$

$$V_f = \text{Min}(|V_f(45, a_1)|, |V_f(-45, a_1)|), \text{ with:}$$

$$\text{total thickness per orientation, } t_{f1} = N_L * t / \text{NoDir} = 1.016$$

$$d_{fv} = d \text{ (figure 11.2, ACI 440)} = 208.00$$

$$f_{fe} ((11-5), \text{ACI 440}) = 259.312$$

$$E_f = 64828.00$$

$$f_e = 0.004, \text{ from (11.6a), ACI 440}$$

$$\text{with } f_u = 0.01$$

$$\text{From (11-11), ACI 440: } V_s + V_f \leq 1.9929E+006$$

$$b_w = 3000.00$$

Calculation of Shear Strength at edge 2, $V_{r2} = 1.2338E+006$

$$\text{From (22.5.1.1) and 11.5.4.3, ACI 318-14: } V_{r2} = V_n < 0.83 * f_c' ^{0.5} * h * d$$

NOTE: In expression (22.5.1.1) ' V_w ' is replaced by ' $V_w + f * V_f$ '

where V_f is the contribution of FRPs (11.3), ACI 440).

From Table (11.5.4.6(d-e)), ACI 318-14: $V_c = 815502.805$

$$\mu_u / \nu_u - l_w / 2 = 0.00 \leq 0$$

$$= 1 \text{ (normal-weight concrete)}$$

$$f_c' = 25.00, \text{ but } f_c' ^{0.5} \leq 8.3 \text{ MPa (22.5.3.1, ACI 318-14)}$$

$$h = 3000.00$$

$$d = 200.00$$

$$l_w = 250.00$$

$$\mu_u = 4.5474513E-012$$

$$\nu_u = 1.1832914E-030$$

$$N_u = 27514.027$$

$$\text{From (11.5.4.8), ACI 318-14: } V_s = V_{s1} + V_{s2} + V_{s3} = 314159.265$$

Vs1 = 157079.633 is calculated for pseudo-Column 1, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs1 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs2 = 157079.633 is calculated for pseudo-Column 2, with:

$$d = 200.00$$

$$A_v = 157079.633$$

$$s = 100.00$$

$$f_y = 500.00$$

Vs2 has been multiplied by 1 ($s < d/2$, according to ASCE 41-17,10.3.4)

Vs3 = 0.00 is calculated for web, with:

$$d = 200.00$$

$$A_v = 0.00$$

$$s = 200.00$$

$$f_y = 500.00$$

Vs3 has been multiplied by $2(1-s/d)$ ($s > d/2$, according to ASCE 41-17,10.3.4)

$$2(1-s/d) = 0.00$$

$$V_f ((11-3)-(11.4), \text{ACI 440}) = 109599.773$$

$$f = 0.95, \text{ for fully-wrapped sections}$$

$wf/sf = 1$ (FRP strips adjacent to one another).

In (11.3) $\sin + \cos$ is replaced with $(\cot + \cot a) \sin a$ which is more a generalised expression, where a is the angle of the crack direction (see KANEPE).

This later relation, considered as a function $Vf(\theta)$, is implemented for every different fiber orientation a_i , as well as for 2 crack directions, $\theta = 45^\circ$ and $\theta = -45^\circ$ to take into consideration the cyclic seismic loading.

orientation 1: $\theta_1 = \theta_1 + 90^\circ = 90.00$

$Vf = \text{Min}(|Vf(45, \theta_1)|, |Vf(-45, \theta_1)|)$, with:

total thickness per orientation, $tf_1 = NL \cdot t / \text{NoDir} = 1.016$

$dfv = d$ (figure 11.2, ACI 440) = 208.00

ffe ((11-5), ACI 440) = 259.312

$Ef = 64828.00$

$fe = 0.004$, from (11.6a), ACI 440

with $fu = 0.01$

From (11-11), ACI 440: $Vs + Vf \leq 1.9929E+006$

$bw = 3000.00$

End Of Calculation of Shear Capacity ratio for element: wall W1 of floor 1

At local axis: 2

Start Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (d)

Section Type: rcrws

Constant Properties

Knowledge Factor, $\phi = 0.85$

According to 10.7.2.3, ASCE 41-17, shear walls with

transverse reinforcement percentage, $\rho_n < 0.0015$

are considered Force-Controlled and lower-bound strengths are used according to 7.5.1.3, ASCE 41-17

$\rho_n = 0.0010472$

with $\rho_n = \rho_{s1} + \rho_{s2} + \rho_{s3}$, being the shear reinf. ratio in a plane perpendicular to the shear axis 2

(pseudo-col.1 $\rho_{s1} = As1 \cdot b1 / s1 = (As1 \cdot h1 / s1) / Ac = 0.0005236$

$h1 = 250.00$

$s1 = 100.00$

total area of hoops perpendicular to shear axis, $As1 = 157.0796$

(pseudo-col.2 $\rho_{s2} = As2 \cdot b2 / s2 = (As2 \cdot h2 / s2) / Ac = 0.0005236$

$h2 = 250.00$

$s2 = 100.00$

total area of hoops perpendicular to shear axis, $As2 = 157.0796$

(grid $\rho_{s3} = As3 \cdot b3 / s3 = (As3 \cdot h3 / s3) / Ac = 0.00$

$h3 = 250.00$

$s3 = 200.00$

total area of hoops perpendicular to shear axis, $As3 = 0.00$

total section area, $Ac = 750000.00$

Consequently:

New material of Secondary Member: Concrete Strength, $f_c = f_{c_lower_bound} = 25.00$

New material of Secondary Member: Steel Strength, $f_s = f_{s_lower_bound} = 500.00$

Concrete Elasticity, $E_c = 26999.444$

Steel Elasticity, $E_s = 200000.00$

Total Height, $H_{tot} = 3000.00$

Edges Width, $W_{edg} = 250.00$

Edges Height, $H_{edg} = 600.00$

Web Width, $W_{web} = 250.00$

Cover Thickness, $c = 25.00$

Element Length, $L = 3000.00$

Secondary Member

Ribbed Bars

Ductile Steel

Without Detailing for Earthquake Resistance (including stirrups not closed at 135°)

Longitudinal Bars With Ends Lapped Starting at the End Sections

Adequate Lap Length ($l_b / l_d \geq 1$)

FRP Wrapping Data

Type: Carbon

Cured laminate properties (design values)

Thickness, $t = 1.016$

Tensile Strength, $f_{fu} = 1055.00$

Tensile Modulus, $E_f = 64828.00$

Elongation, $\epsilon_{fu} = 0.01$

Number of directions, $N_{oDir} = 1$

Fiber orientations, $b_i = 0.00^\circ$

Number of layers, $N_L = 1$

Radius of rounding corners, $R = 40.00$

Stepwise Properties

Bending Moment, $M = 1.5868888E-010$

Shear Force, $V_2 = -6.5495990E-014$

Shear Force, $V_3 = 20258.761$

Axial Force, $F = -30990.641$

Longitudinal Reinforcement Area Distribution (in 2 divisions)

-Tension: $A_{st} = 0.00$

-Compression: $A_{sc} = 6346.017$

Longitudinal Reinforcement Area Distribution (in 3 divisions)

-Tension: $A_{st,ten} = 2368.761$

-Compression: $A_{st,com} = 2368.761$

-Middle: $A_{st,mid} = 1608.495$

Mean Diameter of Tension Reinforcement, $D_{bL} = 17.20$

New component: From table 7-7, ASCE 41_17: Final chord rotation Capacity $u_R = 1.0^*$ $u = 0.01683488$

$u = y + p = 0.01683488$

- Calculation of y -

$y = (M_y \cdot I_p) / (E I)_{Eff} = 0.00183488$ ((10-5), ASCE 41-17))

$M_y = 2.8221E+008$

$(E I)_{Eff} = 0.35 \cdot E_c \cdot I$ (table 10-5)

$E_c \cdot I = 1.0547E+014$

$I_p = 0.5 \cdot d = 0.5 \cdot (0.8 \cdot h) = 240.00$

Calculation of Yielding Moment M_y

Calculation of y and M_y according to Annex 7 -

$y = \text{Min}(y_{ten}, y_{com})$

$y_{ten} = 1.5745713E-005$

with $f_y = 500.00$

$d = 208.00$

$y = 0.23666646$

$A = 0.01026923$

$B = 0.00621105$

with $p_t = 0.00379609$

$p_c = 0.00379609$

$p_v = 0.00257772$

$N = 30990.641$

$b = 3000.00$

$\alpha = 0.20192308$

$y_{comp} = 3.3999485E-005$

with f_c^* (12.3, (ACI 440)) = 25.002

$f_c = 25.00$

$f_l = 0.17503396$

$b = 3000.00$

$h = 250.00$
 $A_g = 750000.00$
 From (12.9), ACI 440: $k_a = 0.00364754$
 $g = p_t + p_c + p_v = 0.0101699$
 $rc = 40.00$
 $A_e/A_c = 0.52524587$
 Effective FRP thickness, $t_f = NL * t * \cos(\theta_1) = 1.016$
 effective strain from (12.5) and (12.12), $\epsilon_{fe} = 0.004$
 $f_u = 0.01$
 $E_f = 64828.00$
 $E_c = 26999.444$
 $y = 0.23569848$
 $A = 0.01002092$
 $B = 0.00611172$
 with $E_s = 200000.00$

Calculation of ratio I_b/I_d

Adequate Lap Length: $I_b/I_d \geq 1$

- Calculation of p -

Considering wall controlled by flexure (shear control ratio ≤ 1),
 from table 10-19: $p = 0.015$
 with:

- Condition i (shear wall and wall segments)
 $-(A_s - A_s') * f_y + P) / (t_w * l_w * f_c') = -0.16757429$

$A_s = 0.00$
 $A_s' = 6346.017$
 $f_y = 500.00$
 $P = 30990.641$
 $t_w = 3000.00$
 $l_w = 250.00$
 $f_c = 25.00$

- $V / (t_w * l_w * f_c^{0.5}) = 2.1033264E-019$, NOTE: units in lb & in

- Confined Boundary: No

Boundary hoops spacing does not exceed $8d_b$ ($s_1 < 8 * d_b$ and $s_2 < 8 * d_b$)

Boundary Trans. Reinf. does not exceed 50% of ACI 318 provision ($V_{w1} + V_{w2} > 0.50 * (V - V_c - V_{w3})$)

With

Boundary Element 1:

$V_{w1} = 157079.633$
 $s_1 = 100.00$

Boundary Element 2:

$V_{w2} = 157079.633$
 $s_2 = 100.00$

Grid Shear Force, $V_{w3} = 0.00$

Concrete Shear Force, $V_c = 183178.256$

(The variables above have already been given in Shear control ratio calculation)

Mean diameter of all bars, $d_b = 17.33333$

Design Shear Force, $V = 6.5495990E-014$

End Of Calculation of Chord Rotation Capacity for element: wall W1 of floor 1

At local axis: 3

Integration Section: (d)