



# Dynamic Soil-Structure Interaction

- With three case studies: ➤ Silo Building Zürich (118 Meter)
- Azadi Hotel Tehran (86 Meter)
  - MIT Green Building (83 Meter)

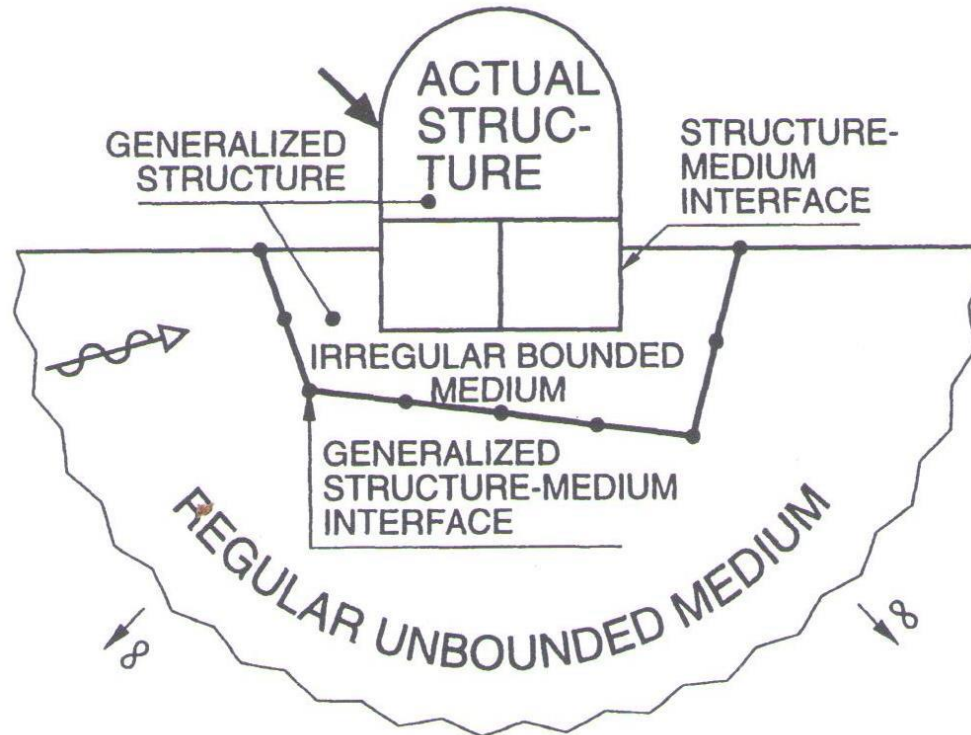
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Distinguished speaker at M.I.T., Civil Engineering Department, Cambridge, USA

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27th September 2015



## Problem definition of dynamic unbounded medium-structure-interaction analysis

# Case Studies

Silo Zürich 118 Meter

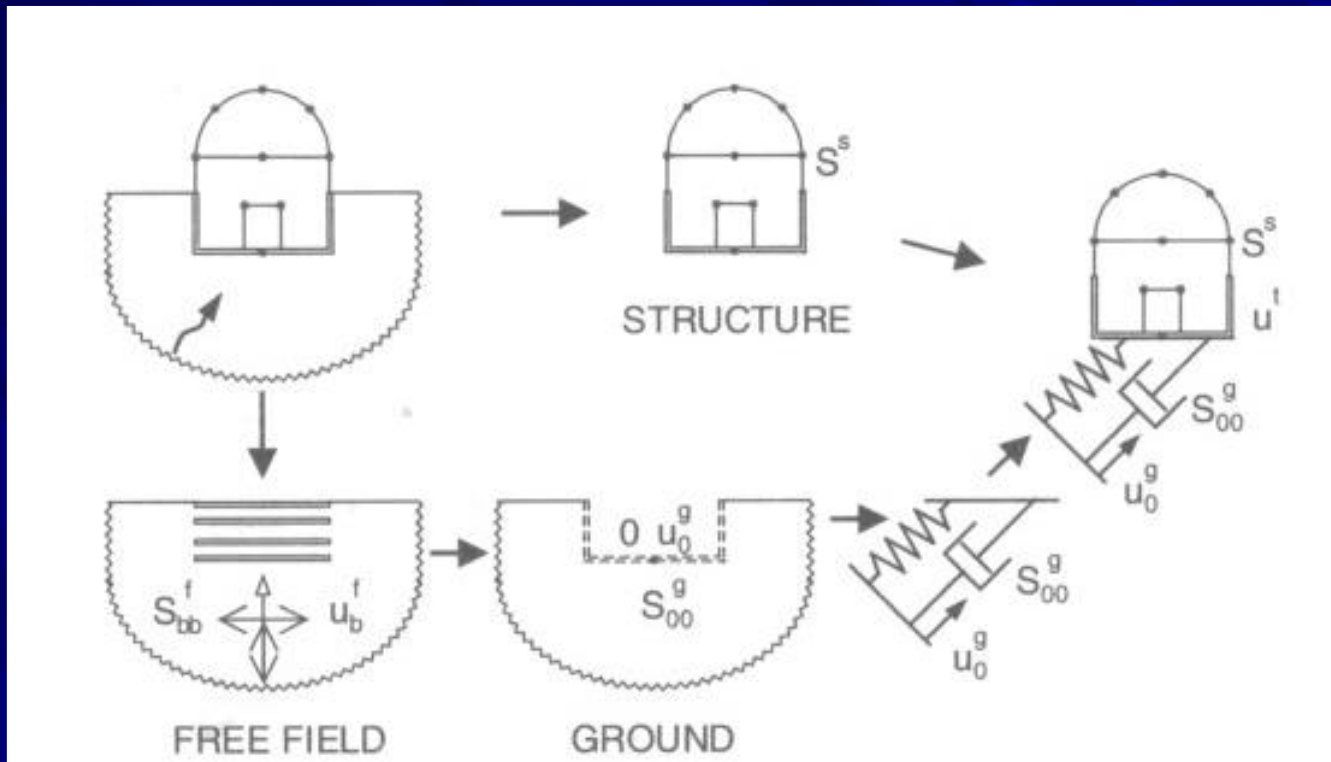


Azadi Hotel Tehran 86 Meter



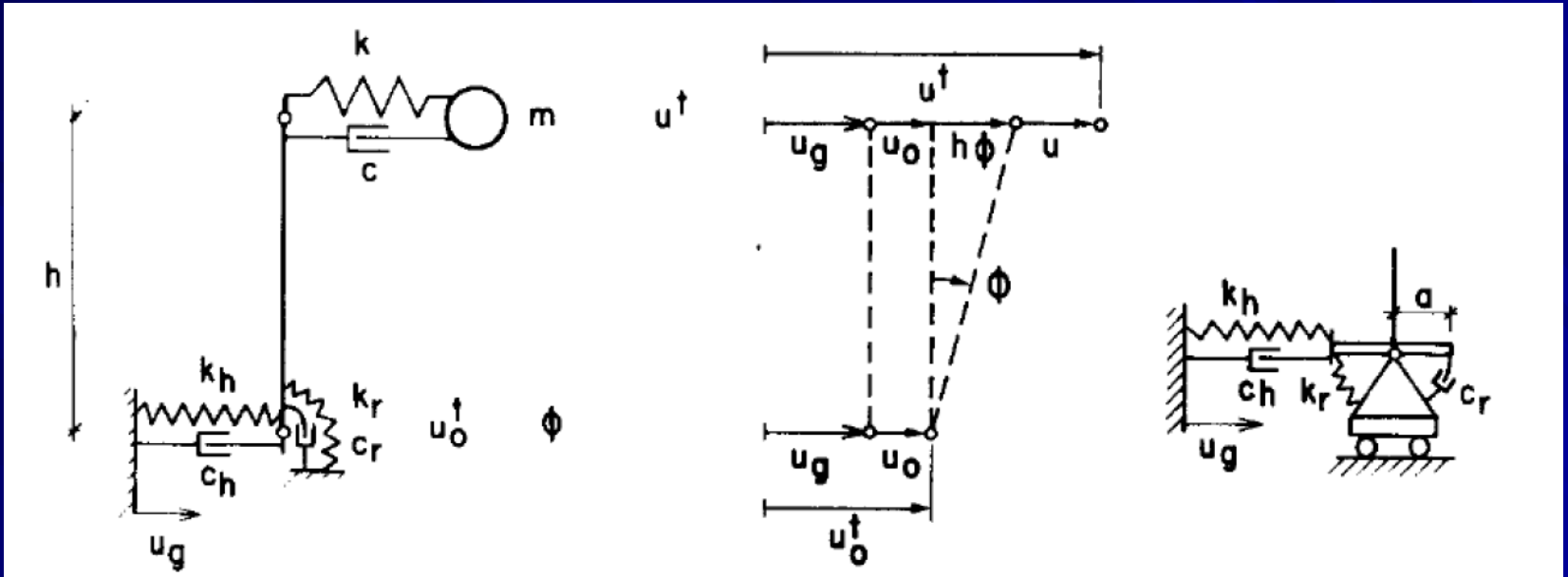
MIT Green Building 83 Meter





Substructure method: Physical interpretation of basic equations of motion in total displacements with effective foundation input motion





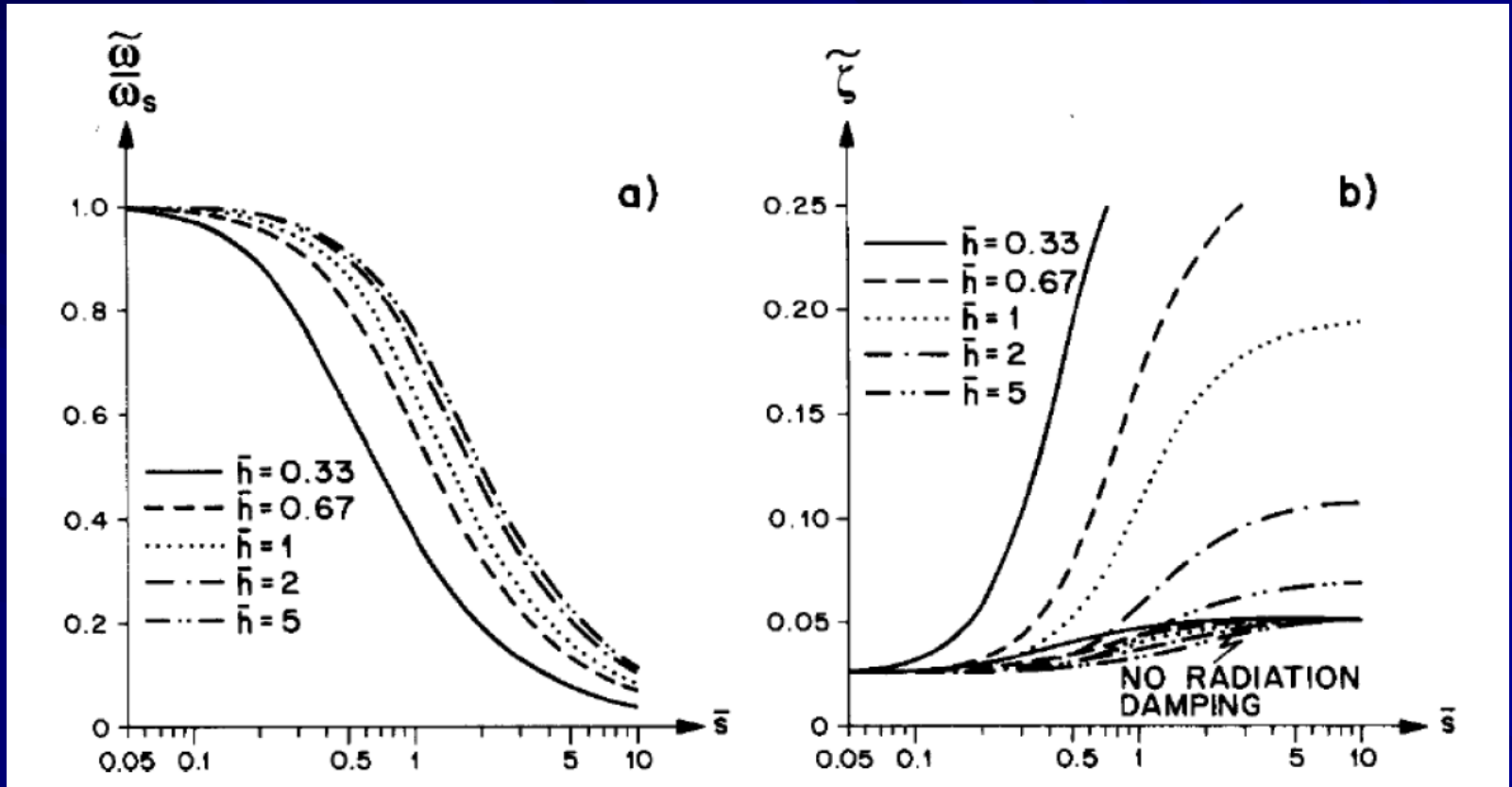
Model with one dynamic degree of freedom

<b>Structural displacement</b>	<b>8 cm</b>
<b>Soil displacement</b>	<b>2 cm</b>
<b>Displacement due to rocking</b>	<b>11</b>
<b>Total displacements</b>	<b>21 cm</b>

Silo Zürich, roof displacements in cm, Swiss SIA earthquake

<b>Structural displacement</b>	<b>20 cm</b>
<b>Soil horizontal displacement</b>	<b>2 cm</b>
<b>Displacement due to rocking</b>	<b>8</b>
<b>Total displacements</b>	<b>30 cm</b>

MIT-Building 54, roof displacement in cm, Boston earthquake



Properties of equivalent one-degree-of-freedom system,  
varying slenderness ratio

## Example: Determination of equivalent frequency for MIT Green Building

$$\bar{s} = \frac{\omega_s h}{c_s} = \frac{2\pi}{1.3978} \times \left( \frac{2}{3} \times 83.7 \right) = 1.2$$

$$\bar{h} = \frac{h}{a} = \frac{h}{\sqrt{\frac{2L \times 2B}{\pi}}} = \frac{2/3 \times 83.7}{\sqrt{\frac{34 \times 20.5}{\pi}}} = 3.74$$

$$\bar{m} = \frac{m}{\rho a^3} = \frac{12238 \times 1000}{2034 \times \left( \sqrt{\frac{34 \times 20.5}{\pi}} \right)^3} = 1.82$$

$$\frac{\tilde{\omega}^2}{\omega_s^2} = \frac{1}{1 + \frac{\bar{m}\bar{s}^2}{8} \left[ \frac{2-\nu}{\bar{h}^2} + 3(1-\nu) \right]} = 0.589 \Rightarrow \tilde{\omega} = \sqrt{0.589} \times \frac{2\pi}{1.3978} = 3.45 \text{ Hz}$$

$$\zeta_{eq} = \frac{\tilde{\omega}^2}{\omega_s^2} \zeta + \left( 1 - \frac{\tilde{\omega}^2}{\omega_s^2} \right) \zeta_g + \frac{\tilde{\omega}^3}{\omega_s^3} \frac{\bar{s}^3 \bar{m}}{\bar{h}} \left[ 0.036 \frac{2-\nu}{\bar{h}^2} + 0.028(1-\nu) \right] \approx 0.055$$



## Determination of soil dynamic stiffness: dampers, springs

### a) Rigorous Models

- FEM
- Boundary Element Method, BEM
- SBFEM

### b) Approximate Models

- Cone
- Lumped Mass Model

# Overview of Zürich City

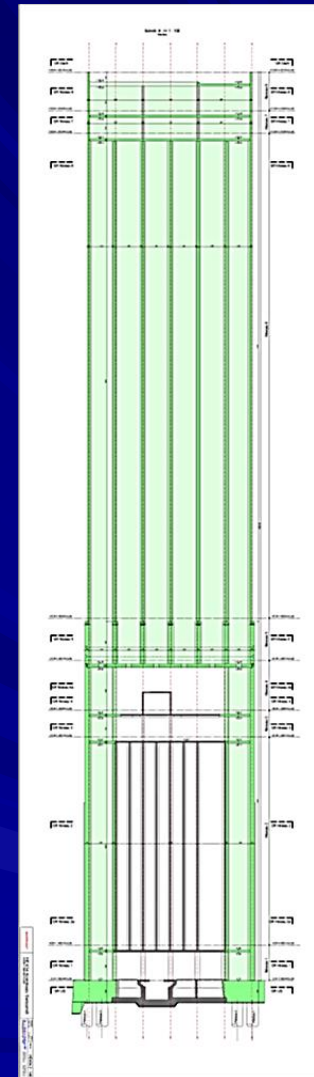


Height	118 m		
Floorplan dimensions	22.40 m x 30.0 0m		
Cubic	81'430 m <sup>3</sup>		
Roof displacement	direction west – east direction north – south	± 17 cm ± 21 cm	
First eigenfrequency		0.32 HZ	
Excavation volume	with piles	5'000 m <sup>3</sup>	
Piles	49	diameter 1.50 m	length 40 – 45 m
Beton volume		18'000 m <sup>3</sup>	
Steel volume		2'700'000 kg	

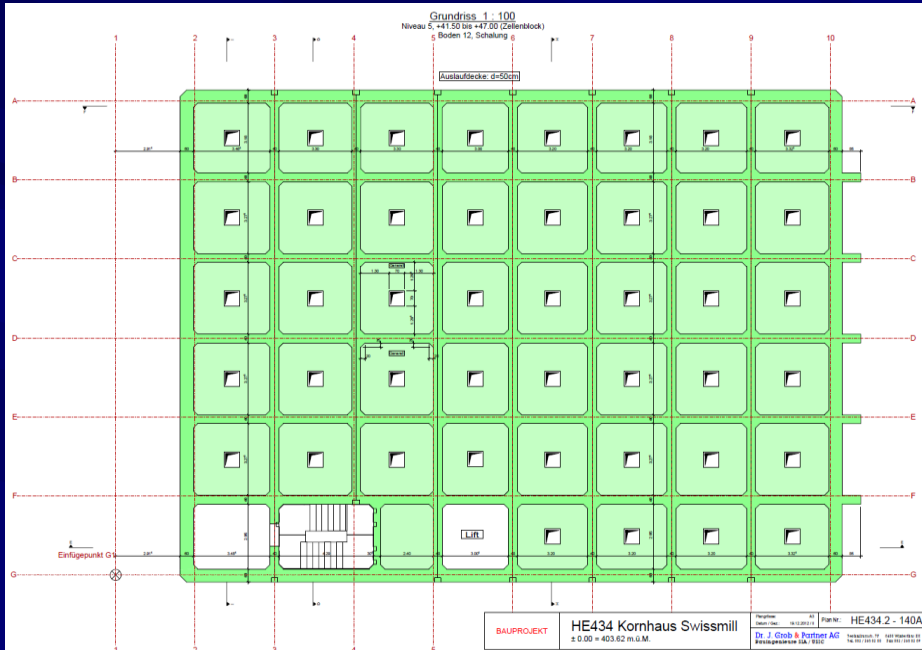
### Technical data of Silo in Zürich



Existing silo: 38 meter high  
50 years old

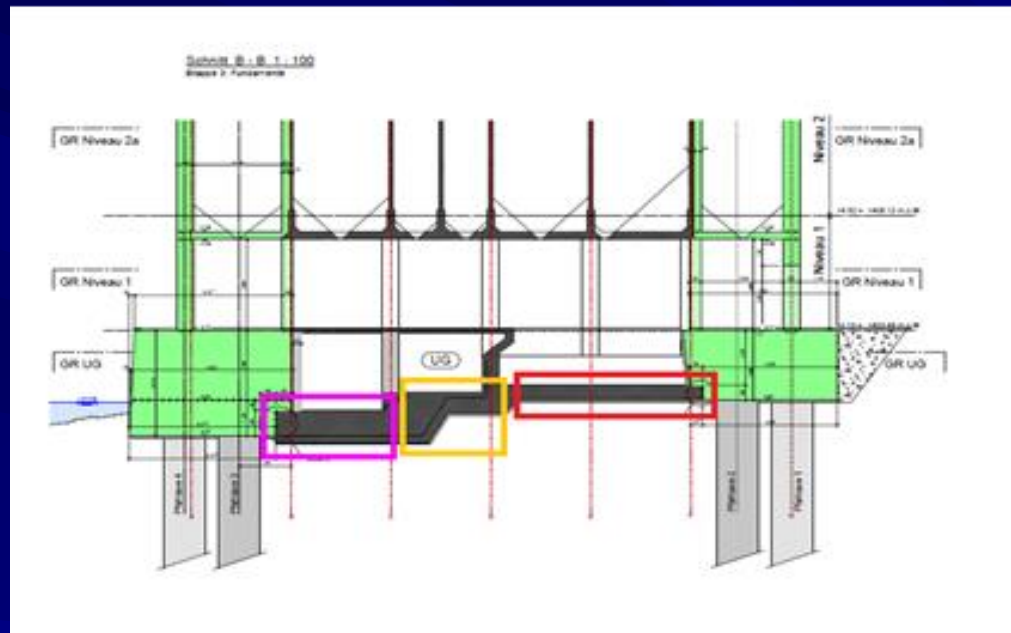


Existing silo (gray)  
New silo (green)



Typical floor plan view

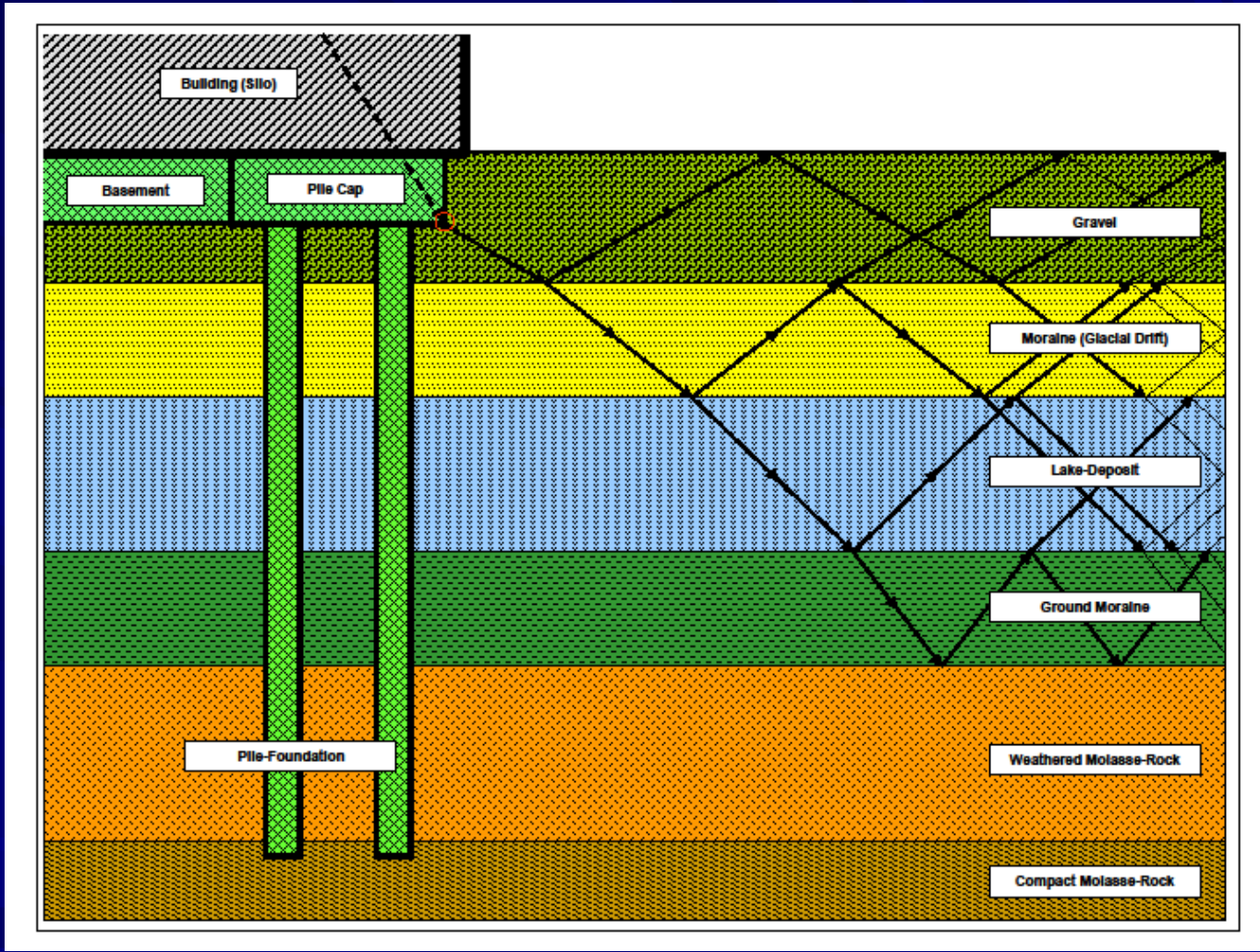




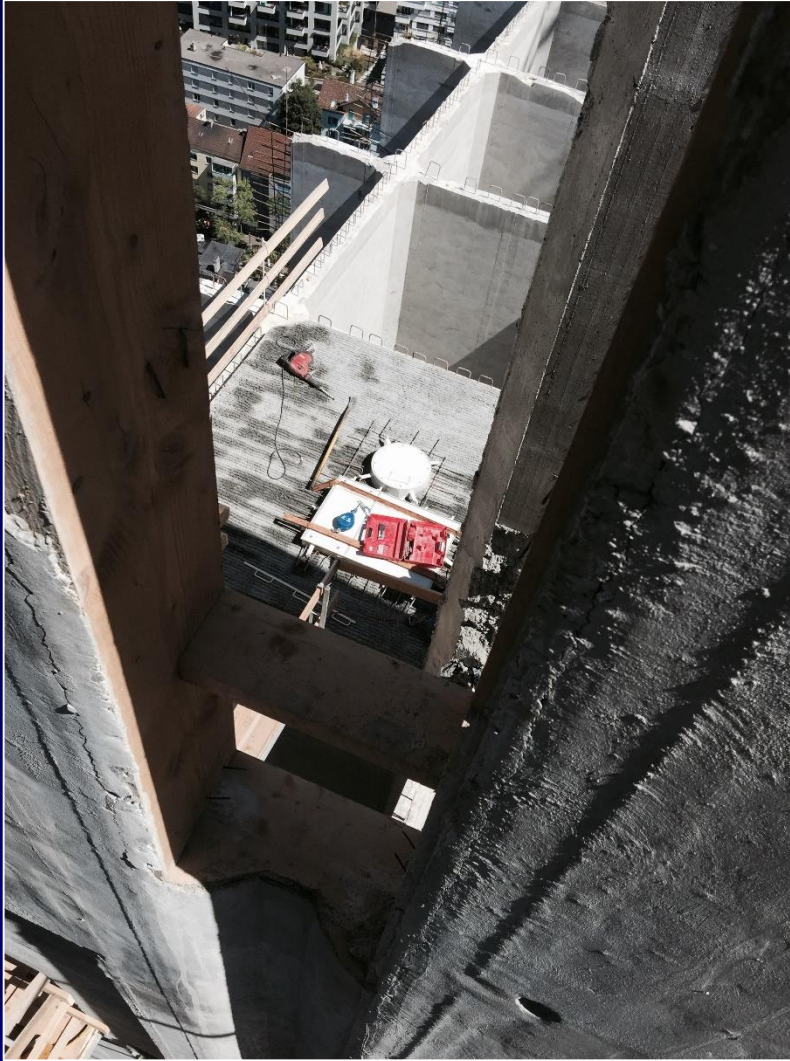
Foundation of the existing  
and of the new silo

Existing silo: Mat foundation

New silo: Pile foundation with  
49 piles and two pile  
cops 4.2 Meter



Site layers and the wave pattern



- Sight from top of silo to the ground
- Distribution of silo cells, 118 Meter
- Construction stage, August 2015
- Construction work at severe conditions, safety net and ropes





Zürich Riverbank, August 2015



- Reinforcement
- Placing the reinforcement at 110 Meter
- 24 hours work (3 shifts, 8 hours)
- 81 Meter at in-situ concrete in 21 days
- 4.1 Meter per 24 hours





Top of silo, August 2015

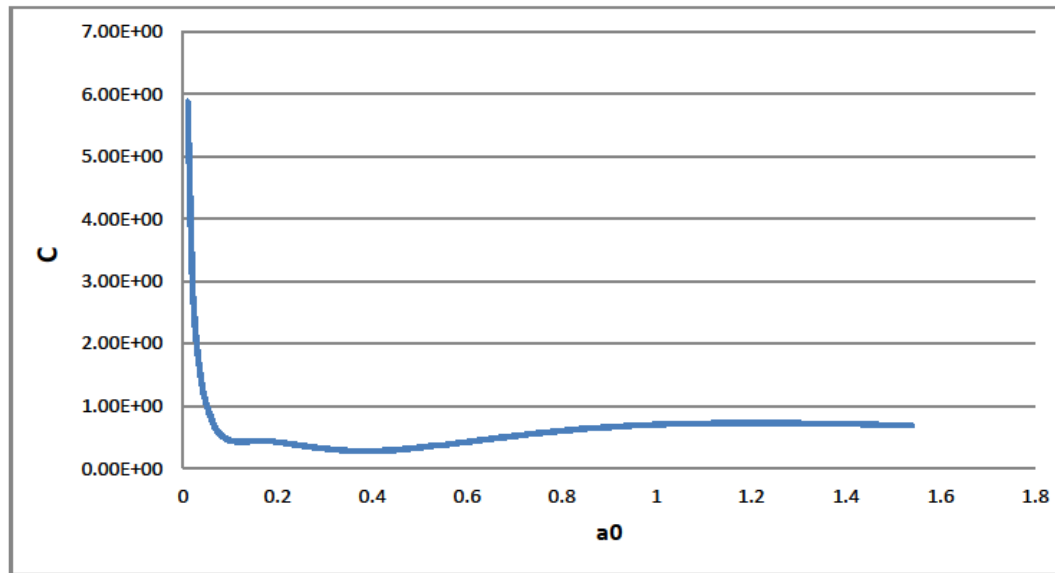


Professor Ahmed Ghoniem MIT, Dr. Sassan Mohasseb, Zürich 2015





Photo from top of silo:  
River with train viadukt



Soil Damping calculated with CONAN using Cone Models (Wolf, ETH)

## References

1. S. Mohasseb, Seismic analyses of steel moment resisting frames, Stanford University, Engineer thesis, Stanford CA, USA, 1980.
2. S.K. Mohasseb and Wolf, J.P., Recursive evaluation of interaction forces of unbounded soil in frequency domain. *Soil Dyn. Earthq. Eng.* 8(4), 176-188, 1989.
3. S. Mohasseb, Non-linear seismic analysis of fully base isolated structures on flexible soils. Thesis ETHZ No. 8454, Zurich 1988.
4. K.-J. Bathe, *Finite Element Procedures*. Prentice Hall, Pearson Education, Inc. USA, (2006).
5. J.P. Wolf, von Arx, G.A., de Barros, F.C.P. and Kakubo, M. Seismic analysis of the pile foundation of the reactor building of the NPP Angra 2, *Nucl. Eng. Des.* 65(3), 329-34, 1981.
6. J.P. Wolf, *Scaled Boundary Finite Element Method*. John Wiley, Chicester England (2003).
7. J.P. Wolf and Deeks, A.J., *Foundation Vibration Analysis: A strength-of-materials approach*. Elsevier Oxford OX2 8DP (2004).
8. A.M. Kaynia, Dynamic stiffness and seismic response of pile groups. Research Report R82-03, Dept. Civil Eng., M.I.T. Cambridge, Massachusetts, 1982.
9. A.M. Kaynia, Dynamic response of pile foundations with flexible slabs. *Earthquakes and Structures*, 3(3-4), 495-506, 2012.
10. E. Kausel, M.I.T. Lectures on vibration, Cambridge MA, USA, November 2013.
11. T. Hughes, Stanford lecture notes on finite element techniques, Stanford University, Stanford CA, USA, 1981.